

D01) GENERAL

D01-1 GENERAL INFORMATION

- THE INFORMATION PRESENTED ON THESE DRAWINGS HAS BEEN DESIGNED AND ANALYZED IN ACCORDANCE WITH THE 2012 ONTARIO BUILDING CODE & 2010 NATIONAL BUILDING CODE OF CANADA. CONSTRUCTION IS TO BE PERFORMED IN ACCORDANCE WITH THIS AND ALL OTHER APPLICABLE CODES.
 - 1. CONCRETE STRUCTURE DESIGNED IN ACCORDANCE WITH CSA A23.3-04 (R2010)
 - 1.2 STEEL STRUCTURE DESIGNED IN ACCORDANCE WITH CANCSA S16-09
 - 1.3 WOOD STRUCTURE DESIGNED IN ACCORDANCE WITH CANCSA S08-105
 - 1.4 MASONRY STRUCTURE DESIGNED IN ACCORDANCE WITH CANCSA S304-10(6)
- GUARDRAILS/HANDRAILS SHALL BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH 1.4.1.15, 3.4.1.4 AND 3.4.1.5 OF THE 2012 ONTARIO BUILDING CODE/2010 NATIONAL BUILDING CODE. STAMPED SHOP DRAWINGS TO BE SUBMITTED. IN ADDITION, GLASS IN GUARDS SHALL COMPLY WITH "SUPPLEMENTARY STANDARD 58-13".
- THE ROOF HAS BEEN DESIGNED FOR THE REQUIRED STORM WATER FLOW RESTRICTION IN ACCORDANCE WITH 2012 ONTARIO BUILDING CODE/2010 NATIONAL BUILDING CODE REQUIREMENTS.
- CONTRACTOR IS TO VERIFY/COORDINATE ALL DIMENSIONS/PENETRATIONS WITH ARCHITECTURAL/MECHANICAL/ELECTRICAL DRAWINGS PRIOR TO CONSTRUCTION. REPORT INCONSISTENCIES BEFORE PROCEEDING WITH WORK. ANY OPENINGS NOT INDICATED ON STRUCTURAL DRAWINGS ARE TO BE APPROVED BY STRUCTURAL ENGINEER IN WRITING PRIOR TO CONSTRUCTION.
- CADD VERSIONS OF THE STRUCTURAL DRAWINGS SHALL BE MADE AVAILABLE TO THE CONTRACTOR UPON THE COMPLETION OF A RELEASE FORM INDEMNIFYING THE CONSULTANT FROM ANY ERRORS OR OMISSIONS ASSOCIATED WITH THE CADD FILES.
- LADDERS SHALL BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN PART 4 AND PART 3 OF THE 2012 ONTARIO BUILDING CODE/2010 NATIONAL BUILDING CODE. STAMPED SHOP DRAWINGS TO BE SUBMITTED.
- STEEL STAIRS SHALL BE DESIGNED AND CERTIFIED BY THE FABRICATOR'S PROFESSIONAL ENGINEER LICENSED IN ONTARIO IN ACCORDANCE WITH LOADS PROVIDED IN PART 4 AND PART 3 OF THE 2012 ONTARIO BUILDING CODE/2010 NATIONAL BUILDING CODE. STAMPED SHOP DRAWINGS TO BE SUBMITTED.
- DEMOLITION DETAILS THAT AFFECT THE STRUCTURAL ELEMENTS HAVE BEEN REVIEWED IN ACCORDANCE WITH PARTS 2.4.1.0 AND 11.1 OF THE 2012 ONTARIO BUILDING CODE, WHERE REQUIRED, SUPPLEMENTARY/TEMPORARY/REMEDIAL FRAMING HAS BEEN PROVIDED.
- SEISMIC RESTRAINT OF ARCH/MECHANICAL ELEMENTS NOT NOTED ON THE DRAWINGS ARE THE RESPONSIBILITY OF THE CONTRACTOR'S ENGINEER. RESTRAINT DETAILS ARE TO BE DEVELOPED IN ACCORDANCE WITH THE 2012 OBC/2010 NBC. CONTRACTOR'S ENGINEER IS RESPONSIBLE FOR THE DESIGN AND DETAILING OF SEISMIC RESTRAINTS AND ISOLATIONS AS REQUIRED BY SPECIFICATIONS INCLUDING THE VERIFICATION THAT THE EXISTING/NEW STRUCTURE IS CAPABLE OF SAFELY SUPPORTING THE IMPOSED LOADS IN ACCORDANCE WITH THE 2012 OBC/2010 NBC. NO ELEMENTS MAY BE CONSTRUCTED WITHOUT WRITTEN CONFIRMATION OF THESE CONDITIONS BY CONTRACTOR'S ENGINEER.
- NO FOUNDATION ELEMENTS ARE TO BE CONSTRUCTED UNTIL WRITTEN APPROVAL OF THE BEARING SURFACES AND PRESSURES IS PROVIDED BY A GEOTECHNICAL ENGINEER THROUGH ON-SITE INVESTIGATION. FAILURE TO COMPLETE THIS WORK COULD RESULT IN THE REMOVAL/RESTATEMENT OF ANVIAL FOUNDATION ELEMENTS AT CONTRACTOR'S OWN COST.
- NEW OPENINGS IN EXISTING/DIRECTED CONCRETE/MASONRY ELEMENTS:
 - REFER TO MECHANICAL/ELECTRICAL DWGS FOR LAYOUTS.
 - SCAN TABS OF SLAB/WALL PRIOR TO REMOVALS. MARK OUT OPENING POSITION ON TABS OF SLABS/E. OF WALL.
 - CONTACT ENGINEER FOR REVIEW OF SCANS/MARKS PRIOR TO COMMENCEMENT OF ANY DEMOLITION.
 - NO WORK TO COMMENCE UNTIL RECEIVING WRITTEN APPROVAL FROM ENGINEER.
 - PROVIDE ADEQUATE PROTECTION OF SURROUNDING AREAS DURING REMOVALS.
 - SAW/CUT/CORE CUT OPENINGS AS REQUIRED.
 - DO NOT OVERTCUT.
 - ANY REQUIRED SUPPLEMENTAL STRENGTHENING/REINFORCING IS TO BE ERECTED PRIOR TO REMOVALS.
- CONTRACTOR TO PROVIDE PRE-ENGINEERED SHORING AS REQUIRED TO ACCOMMODATE THE CONTRACTOR'S CONSTRUCTION ACTIVITIES AND TO PREVENT DAMAGE TO ANY ADJACENT PROPERTY. ALL CONSTRUCTION ACTIVITIES TO BE LIMITED TO THE LIMITS OF THE CONSTRUCTION SITE AND ALL DAMAGE TO EXISTING PROPERTIES MUST BE REINSTATED.
- CONTRACTOR IS REQUIRED TO SUBMIT CONDUIT AND SLEEVING SHOP DRAWINGS FOR ALL FLOORS/ROOFS/WALLS/COLUMNS PRIOR TO THE ERECTION/CONSTRUCTION/FABRICATION OF ANY OF THESE ELEMENTS. THE DRAWINGS ARE TO LOCATE/DIMENSION THE CLEAR SIZES OF OPENINGS/SLEEVES/CONDUITS IN PLAN (FLOORS/ROOFS/COLUMNS) AND ELEVATION (WALLS/BEAMS). THE COORDINATION OF THE VARIOUS DISCIPLINES/SUBTRADES TO ENSURE ALL ITEMS ARE CLEARLY INDICATED IS THE SOLE RESPONSIBILITY OF THE CONTRACTOR. DRAWINGS ARE TO BE SUBMITTED A MINIMUM OF 4 WEEKS PRIOR TO THE CONSTRUCTION OF THE AFFECTED ELEMENT.

D01-2 GRAVITY LOADS:

SLS/LS VALUES:

SNOW, I _s :	ULS=1.0 SLS=0.9	SNOW LOAD FACTORS:
WIND, I _w :	ULS=1.0 SLS=0.75	S = I _s [S _w C _w C _e C _d C _a] + S ₀
SEISMIC, I _e :	ULS=1.0	S ₀ = 2.5 kPa
		S ₁ = 0.4 kPa
		C _d = 0.8
		C _w = 1.0
		C _e = 1.0

PITCHED ROOF:	DEAD:	ROOFING:	0.60
	STRUCTURE:	0.25	
	DECK:	0.10	
	CEILING:	0.25	
	MECH./ELEC.:	0.25	
		1.45 kPa	

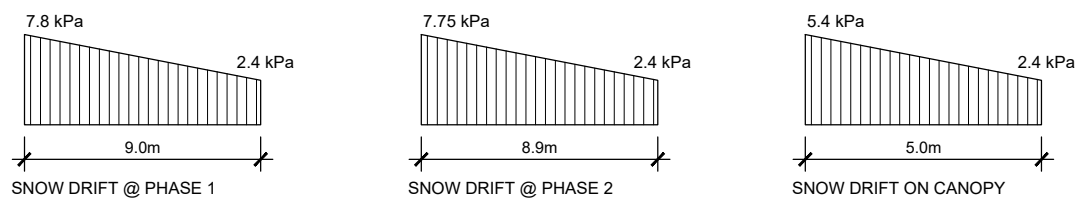
LIVE:	1.0 kPa
SNOW:	2.4 kPa (+ DRIFT) UN

FLAT ROOF:	DEAD:	ROOFING:	0.60
	STRUCTURE:	0.25	
	150 CONC. ON 36 DECK:	4.00	
	CEILING:	0.25	
	MECH./ELEC.:	0.25	
		5.35 kPa	

LIVE:	1.0 kPa
SNOW:	2.4 kPa (+ DRIFT) UN

CANOPY ROOF:	DEAD:	ROOFING:	0.60
	STRUCTURE:	0.25	
	DECK:	0.10	
	MECH./ELEC.:	0.25	
		1.20 kPa	

LIVE:	1.0 kPa
SNOW:	2.4 kPa (+ DRIFT) UN



D01-3 SEISMIC SYSTEM/LOADING DATA

SEISMIC FORCE RESISTING SYSTEM (SFRS)

SFRS: SYSTEM & CONNECTIONS (2012 OBC CLAUSE 4.1.8.9.4 4.1.8.10)
LATERAL LOAD RESISTING SYSTEM: CONVENTIONAL CONSTRUCTION STEEL MOMENT FRAME
R_o = 1.0
R_o = 1.3
CSA STANDARD CANCSA S16-09
APPLICABLE CLAUSE(S): 27.11

SFRS: DIAPHRAGMS & CONNECTIONS (2012 OBC CLAUSE 4.1.8.15)
CSA STANDARD CANCSA S16-09
APPLICABLE CLAUSE(S): 27.11

SFRS: SYSTEM FOUNDATIONS (2012 OBC CLAUSE 4.1.8.16)
CSA STANDARD CANCSA A23.3-04
APPLICABLE CLAUSE(S): 21

CONFIRMATION: FOUNDATIONS HAVE BEEN DESIGNED TO RESIST THE LATERAL FORCES APPLIED TO THE SFRS IN ACCORDANCE WITH THE 2012 OBC INCLUDING ALL APPLICABLE AMPLIFICATION FACTORS.

SEISMIC IMPORTANCE FACTOR (2012 OBC CLAUSE 4.1.8.5)

I_s = 1.0

REFERENCE CITY: ALMONTÉ

SITE CLASS: THE NOTED SITE CLASSIFICATION FOR SEISMIC SITE RESPONSE AND SHEAR STRENGTH PARAMETERS INDICATED ARE AS REPORTED IN THE GEOTECHNICAL REPORT

CLASS 'C': (F_a = 1.0 F_v = 1.0)

PGA: 0.28

RESPONSE SPECTRUM DATA

0% DAMPED SPECTRAL RESPONSE ACCELERATION VALUES FOR REFERENCE CITY (2012 OBC SUPPLEMENTARY STANDARD 58-1)

S ₀ (2)	= 0.55
S ₀ (0.5)	= 0.27
S ₀ (0.1)	= 0.13
S ₀ (0)	= 0.042

DESIGN SPECTRAL RESPONSE ACCELERATION VALUES (DSRAV) (2012 OBC CLAUSE 4.1.8.4)

Fa=1.0:Fv=1.0)	
	= 0.55
	= 0.55
	= 0.27
	= 0.13
	= 0.042
	= 0.021

SYSTEM RESTRICTION VALUE: $\frac{M_F + M_D(2)}{W} \geq 0.55$ ☒ YES ☐ NO

PERIOD DATA

<u>EMPIRICAL PERIOD:</u> (2012 OBC CLAUSE 4.1.8.11.(3) (a),(b)or(c))	
T _{EMPIRICAL, M} =	0.196 sec
T _{EMPIRICAL, E} =	0.196 sec

MODAL PERIOD: (2012 OBC CLAUSE 4.1.8.11 (3) (d) AND 4.1.8.3 (8))

T _{MODAL, M} =	0.513 sec
T _{MODAL, E} =	0.595 sec

DESIGN PERIOD/MODE & MOMENT FACTORS (2012 OBC CLAUSE 4.1.8.11 (5))

S ₀ (2)	= 13.1	S ₀ (0)	= 8.0
S ₀ (0)	= 13.1	S ₀ (0)	= 8.0

T _{DESIGN, M} =	0.294 sec	M _v = 1.0	J _v = 1.0
T _{DESIGN, E} =	0.294 sec	M _v = 1.0	J _v = 1.0

DESIGN FUNDAMENTAL PERIOD BASED DSRAV (2012 OBC CLAUSE 4.1.8.11 (2))

S₀(2) = 0.462
S₀(0) = 0.462

IRREGULARITY REVIEW (2012 OBC CLAUSE 4.1.8.6)

1. VERTICAL STIFFNESS	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO
2. WEIGHT	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO
3. VERTICAL GEOMETRIC	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO
4. IN PLANE DISCONTINUITY	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO
5. OUT OF PLANE	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO
6. WEAK STOREY	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO
7. TORSIONAL	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO
8. NON-ORTHOGONAL	<input type="checkbox"/> YES <input checked="" type="checkbox"/> NO

CONCLUSION: BUILDINGS ARE DYNAMIC ANALYSIS: DYNAMIC PROCEDURE: METHOD: ☐ REGULAR ☒ IRREGULAR ☐ REQUIRED ☒ NOT REQUIRED

MODAL RESPONSE SPECTRUM: ☐ NUMERICAL INTEGRATION TIME HISTORY ☐ N/A

TORSIONAL ECCENTRICITY: ☐ ± 0.10 D₀₄ (4.1.8.11 (10a), B ± 1.7 EQUIV. STATIC FORCE PROCEDURE) ☐ ± 0.10 D₀₄ (4.1.8.11 (10a), B ± 1.7) ☐ ± 0.10 D₀₄ (4.1.8.11 (10a), B ± 1.7, 3-D DYNAMIC ANALYSIS)

STRUCTURAL SEPARATION: ☐ THE ADJACENT STRUCTURES HAVE BEEN SEPARATED IN ACCORDANCE WITH 4.1.8.11 (11) OF THE 2012 O.B.C. ☐ N/A

BUILDING HEIGHT FOR SEISMIC DESIGN: W = 430 MN

BASE SHEAR/MOMENTS:

V_{BASE} = 571 kN (W/R_o) = 0.237 * W = 103 kN

STATIC MAXIMUM/MINIMUM VALUES:

NORTH-SOUTH (+/-): V_{BASE} = 571 kN (W/R_o) = 0.237 * W = 103 kN

EAST-WEST (+/-): V_{BASE} = 571 kN (W/R_o) = 0.237 * W = 103 kN

EAST-WEST (+/-): V_{BASE} = 571 kN (W/R_o) = 0.237 * W = 103 kN

EAST-WEST (+/-): V_{BASE} = 571 kN (W/R_o) = 0.237 * W = 103 kN

EQUIVALENT STATIC (IES) FORCE PROCEDURE (INITIAL SCALING FACTOR)	DYNAMIC ANALYSIS (DYN) PROCEDURE (10)	DESIGN (D) LOADS (1)
2012 OBC CLAUSE 4.1.8.11 (10)	2012 OBC CLAUSE 4.1.8.11 (10)	
NORTH-SOUTH (+/-)	NORTH-SOUTH (+/-)	NORTH-SOUTH (+/-)
V _{BASE} = 571 kN M _{BASE} = 250 kNm	V _{BASE} = 571 kN M _{BASE} = 175 kNm MPMR = 100%	V _{BASE} = 55 kN M _{BASE} = 200 kNm
	NON-ORTHOGONAL EFFECTS HAVE BEEN CONSIDERED IN ACCORDANCE WITH 2012 OBC CLAUSE 4.1.8.8 (c)	<input type="checkbox"/> YES <input checked="" type="checkbox"/> N/A
EAST-WEST (+/-)	EAST-WEST (+/-)	EAST-WEST (+/-)
V _{BASE} = 571 kN M _{BASE} = 250 kNm	V _{BASE} = 46.4 kN M _{BASE} = 142 kNm MPMR = 100%	V _{BASE} = 55 kN M _{BASE} = 200 kNm
	NON-ORTHOGONAL EFFECTS HAVE BEEN CONSIDERED IN ACCORDANCE WITH 2012 OBC CLAUSE 4.1.8.8 (c)	<input type="checkbox"/> YES <input checked="" type="checkbox"/> N/A
NOTES:		
(1) INITIAL DYNAMIC LOAD SCALING FACTOR: S.F. = 1.0, S ₀ = 0.55, S ₁ = 0.27, S ₂ = 0.13, S ₃ = 0.042		
DYNAMIC ANALYSIS PROCEDURE LOADS ARE BASED ON THE EVALUATION OF V ₀₁ AND V ₀₂ IN ACCORDANCE WITH 4.1.8.12 (5) (a) AND (7) OF THE 2012 OBC. LOADS INDICATED SHOW THE DESIGN BASE SHEAR AND CORRESPONDING OVERTURNING MOMENT.		
(3) N/A = NOT USED IN THE DESIGN OF THE BUILDING.		

1.2 WIND:

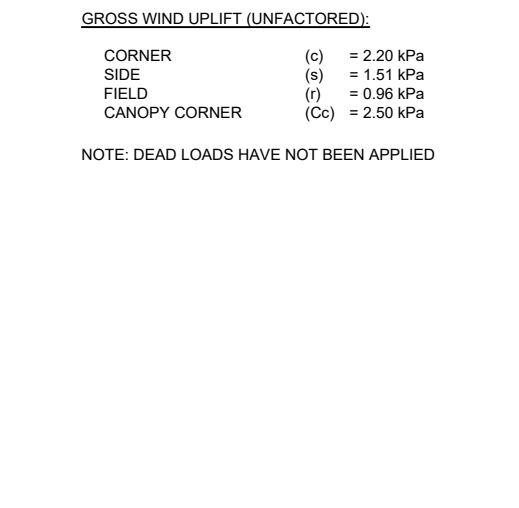
WIND BUILDING:

WIND:	NORTH-SOUTH (+/-)
q ₀ = 0.41 kPa (1 IN 50 YEARS)	V _{WIND} = 17 kN M _{WIND} = 39 kNm
I _w = 1.0 (ULS)	EAST-WEST (+/-)
I _w = 0.75 (SLS)	V _{WIND} = 41 kN M _{WIND} = 92 kNm
C _d C _e = 1.3 (1.95 AT ENDS)	

TYPICAL WALLS: W_{LW} = 0.89 kPa
W_{LE} = 0.98 kPa

GROSS WIND UPLIFT (UNFACTORED):

NOTE: DEAD LOADS HAVE NOT BEEN APPLIED



D01-5 DEFINITIONS:

THE FOLLOWING ABBREVIATIONS HAVE BEEN USED IN THESE NOTES AND DRAWINGS:

ARCH.	AT (SPACING @)	mm	MILLIMETRES
B.	ARCHITECTURAL	MAX.	MAXIMUM
B.L.	BOTTOM LOWER LAYER	MIN.	MINIMUM
B.U.	BOTTOM UPPER LAYER	MECH.	MECHANICAL
C.	CENTRE TO CENTRE	NP	NEAR FACE
C.C.	CENTRE LINE	NTS	NOT TO SCALE
C.O.	CONCRETE	OF	OUTSIDE FACE
C.W.	CORE WALL	PCO	PILE CUT-OFF
E.A.	EACH	PL	PLATE
E.F.	EACH END	SG	STANDARD GALVANIZED
E.S.	EACH SIDE	SW	STEEL WALL
E.W.	EACH WAY	T.	TOP LOWER LAYER
F.F.	FAR FACE	T.U.	TOP UPPER LAYER
HDMR	HEAVY DUTY GALVANIZED TRUSS	TYP.	TYPICAL
I.F.	INSIDE FACE	UL	UPPER LAYER
L.L.	LOWER LAYER	US	UNDERSIDE
M.	METRES	V.	VERTICAL

D01-6 SHOP DRAWINGS

- SUBMIT SHOP DRAWINGS FOR ALL STRUCTURAL WORK AND ANY WORK AFFECTING THE STRUCTURE TO THE CONSTRUCTION MANAGER. OBTAIN ARCHITECT'S & ENGINEER'S APPROVAL BEFORE PROCEEDING WITH THE FABRICATION.
- EACH OF THE FOLLOWING SHOP DRAWINGS MUST BEAR THE SIGNATURE AND STAMP OF A QUALIFIED PROFESSIONAL ENGINEER REGISTERED IN THE PROVINCE (PLUS OTHER DRAWINGS AS NOTED).
 - DRAWINGS FOR ANY TEMPORARY WORK.
 - DRAWINGS FOR ANY STRUCTURAL PARTS DESIGNED BY THE CONTRACTOR'S FORCES INCLUDING EXTERIOR BUILDING ENVELOPE.
 - DRAWINGS FOR STRUCTURAL STEEL JOISTS.
 - FORMWORK.
- SHOP DRAWINGS MUST BE REVIEWED AND STAMPED REVIEWED BY THE CONTRACTOR BEFORE ISSUING TO THE ARCHITECT/ENGINEER. SHOP DRAWINGS NOT STAMPED BY THE CONTRACTOR WILL BE REJECTED. ANY DELAYS IN THE CONSTRUCTION SCHEDULE DUE TO NONCOMPLIANCE WITH THIS REQUIREMENT SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR.
- SUBMIT STRUCTURAL STEEL, STEEL JOIST AND STEEL DECK SHOP DRAWINGS FOR STRUCTURAL ENGINEER'S REVIEW BEFORE FABRICATION. ALL SHOP DRAWINGS SHALL BEAR THE SEAL OF A REGISTERED PROFESSIONAL ENGINEER IN THE PROVINCE OF ONTARIO.
- SHOP DRAWINGS ARE REVIEWED FOR CONFORMANCE WITH THE GENERAL DESIGN CONCEPT. THIS REVIEW DOES NOT IMPLY APPROVAL OF THE DETAILED DESIGN OR QUANTITIES DESCRIBED IN THE SHOP DRAWINGS. THE RESPONSIBILITY FOR THE QUANTITIES AND DETAILED DESIGN OF THE MATERIALS AND COMPONENTS AS REQUIRED TO PROVIDE THE COMPLETE AND SATISFACTORY JOB DESCRIBED IN THE DESIGN DOCUMENTS REMAINS WITH THE CONTRACTOR.

D03) FOUNDATIONS:

D03-1 FOOTINGS:

ALL FOOTINGS TO BEAR ON UNDISTURBED NATIVE MATERIAL WITH MINIMUM ALLOWABLE BEARING STRENGTHS AS NOTED AND AS APPROVED BY GEOTECHNICAL ENGINEER ON SITE. REFERENCE GEOTECHNICAL REPORT.

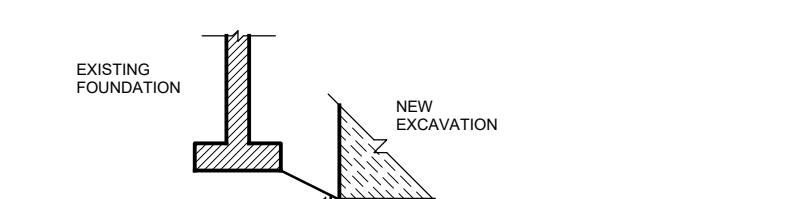
D03-2 EXCAVATION, FOUNDATIONS AND BACKFILL:

REFER TO GEOTECHNICAL REPORT BY:

- PRIOR TO ANY EXCAVATION, VERIFY LOCATION OF EXISTING SERVICES AND TAKE ALL NECESSARY MEASURES TO MAINTAIN SERVICES WHERE REQUIRED. NOTIFY OWNER AND ENGINEER IF ANY SERVICES NOT SHOWN ON PLAN OR OTHERWISE EXPECTED ARE ENCOUNTERED. DO NOT PROCEED FURTHER UNTIL DIRECTED.
- CARE MUST BE TAKEN TO AVOID UNDERMINING EXISTING BUILDING FOUNDATIONS OR UNDERGROUND SERVICES.
- PROTECT SUB-GRADE FROM FREEZING AND FROST ACTION AT ALL TIMES DURING CONSTRUCTION.
- FOOTINGS MUST BEAR ON APPROVED BEARING SURFACES.
- BACKFILL TO WITHIN 200mm OF UNDERSIDE OF SLAB WITH GRANULAR 'B' TYPE II IN LAYERS UP TO 12" THICK, COMPACTED TO MINIMUM 100% SPMD OR AS PER GEOTECHNICAL REPORT.
- FINAL 200mm UNDER SLAB TO BE GRANULAR 'A' COMPACTED TO MINIMUM 100% SPMD OR AS PER GEOTECHNICAL REPORT.
- REUSE OF EXCAVATED GRANULAR MATERIAL IS SUBJECT TO APPROVAL OF GEOTECHNICAL CONSULTANT.

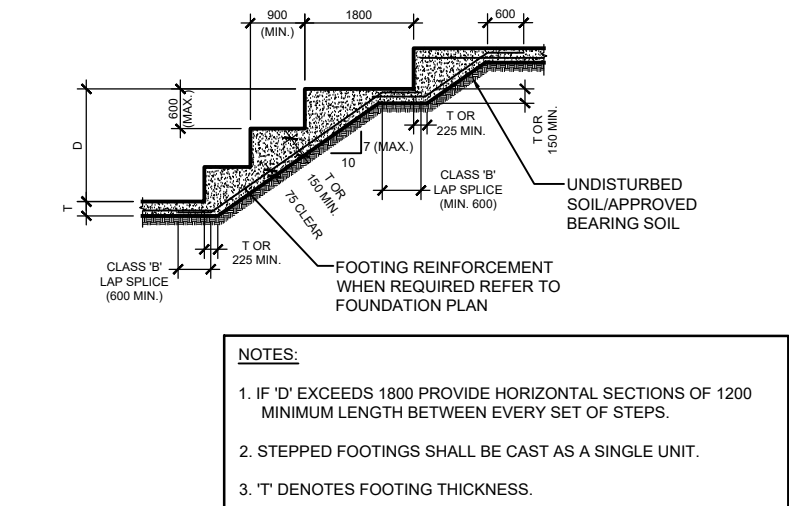
D03-3 PROTECT LATERAL STABILITY OF BEARING STRATA UNLESS NOTED:

UNLESS OTHERWISE OUTLINED IN GEOTECHNICAL REPORT DO NOT EXCAVATE BELOW A LINE EXTENDING DOWNWARD FROM ANY BEARING STRATA AT A SLOPE OF 1 VERTICAL TO 2 HORIZONTAL. ADJUST FOOTING AND TRENCH ELEVATIONS TO MEET THIS REQUIREMENT (SEE DIAGRAM).



D03-4 FOOTINGS STEPS:

LOCATIONS OF FOOTING STEPS TO BE APPROVED BY ENGINEER IN WRITING PRIOR TO CONSTRUCTION.



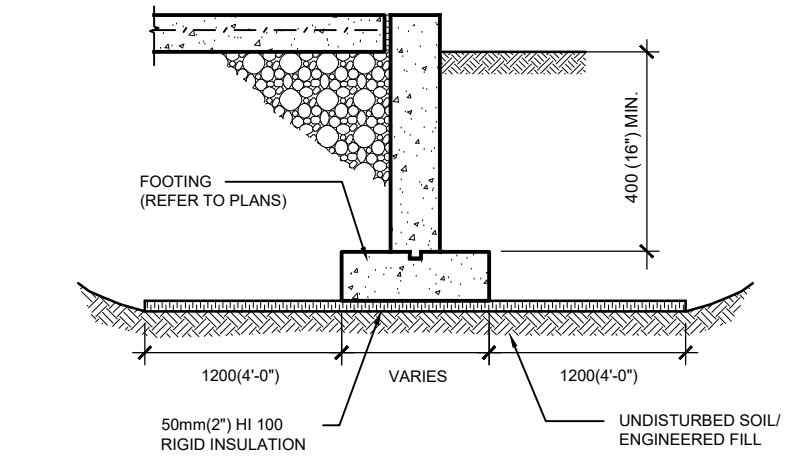
D03-5 MINIMUM FROST COVER REQUIREMENTS:

(NOTE: INCREASE DEPTHS AS REQUIRED BY GEOTECHNICAL REPORT/ENGINEER)

- HEATED BUILDINGS: 1500mm (5'-0")
- HEATED BUILDING (SNOW CLEARED): 1800mm (5'-0")
- ISOLATED AREAS: 2100mm (7'-0")

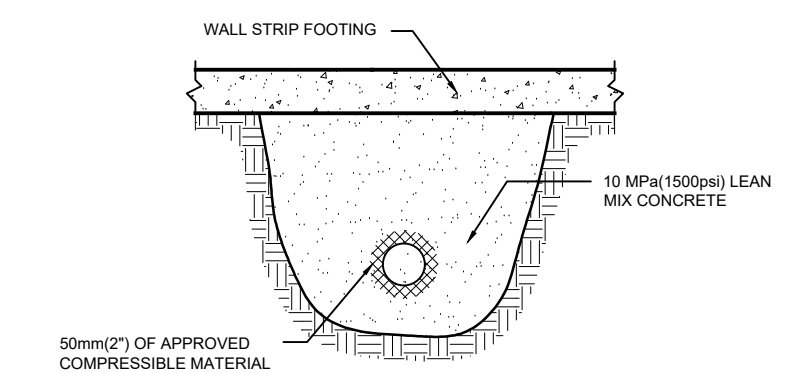
D03-6 ALTERNATE FROST COVER DETAIL:

(NOTE: ONLY TO BE USED AFTER WRITTEN APPROVAL BY ENGINEER WHERE GEOTECHNICAL REPORT REQUIRES MORE SOIL COVER/EXTENTS OF INSULATION. SOILS REPORT GOVERNS)



D03-7 PIPE CROSSING BELOW STRIP FOOTING:

(NOTE: LOCATIONS WHERE PIPES CROSS BELOW FOOTINGS ARE TO BE APPROVED BY ENGINEER IN WRITING PRIOR TO CONSTRUCTION. ENGINEER RESERVES THE RIGHT TO RELOCATE PIPES AS REQUIRED OR LOWER FOOTINGS TO SUIT.)



D03) CONCRETE

D03-1 CONCRETE COVER (CLEAR TO REINFORCING):

US FOOTINGS, PILE CAPS, GRADE BEAMS (AGAINST SOIL)	75mm (3")
FOOTINGS, PILE CAPS, GRADE BEAMS (SIDES & TOP)	50mm (2")
WALLS	40mm (1 1/2")
SLABS	25mm (1") UN
BEAMS	40mm (1 1/2") TO STIRRUPS
COLUMNS	40mm (1 1/2") TO TIES
BALCONIES	40mm (1 1/2") TO TOP STEEL

PROVIDE 32mm (1 1/2") COVER FOR BOTTOM STEEL FOR SLAB ABOVE 3HR, FIRE RATED AREAS. PROVIDE 50mm (2") COVER FOR COLUMN TIES 3HR, FIRE RATED AREAS.

D03-2 SLAB REINFORCING STEEL:

SPACING OF BARS SHALL BE APPROXIMATELY UNIFORM WITHIN THE CORRESPONDING STRIPS. DO NOT ELIMINATE OR DISPLACE REINFORCEMENT TO ACCOMMODATE HARDWARE IF INSERTS CANNOT BE LOCATED AS SPECIFIED. OBTAIN APPROVAL OF ALL MODIFICATIONS FROM ENGINEER BEFORE THE PLACING OF CONCRETE.

WHERE TENSION LAPS ARE SPECIFIED, LAP REINFORCING STEEL IN ACCORDANCE WITH THE REQUIREMENTS OF CAN2-A23.3, LATEST EDITION, ALL OTHER LAPS AND EMBEDMENT OF DOVELS SHALL BE 24 BAR DIAMETERS, BUT NOT LESS THAN 600mm IF NOT SPECIFIED OTHERWISE. WIRE MESH LAPS SHALL BE 150mm MINIMUM.

TYPICAL REBAR REQUIREMENTS:

LAPS: AS ON DRAWINGS

≥ 36 BAR DIA. ≥ 1.5 #4

≥ 600mm (24")

BAR DESIGNATION IN SLABS:

METRIC: 10-15M 6400x4100 T MEANS 10 BARS, SIZE 15M, TOP OF SLAB, 5-6400 LONG / 5-4100 LONG ALTERNATING (+ HOOK LENGTH FOR TOP BARS @ SLAB EDGES)

IMPERIAL: 10-15M 150/08 MEANS 10 BARS, SIZE 15M, TOP OF SLAB, 5-15'-0" LONG / 5-8'-0" LONG ALTERNATING (+ HOOK LENGTH FOR TOP BARS @ SLAB EDGES)

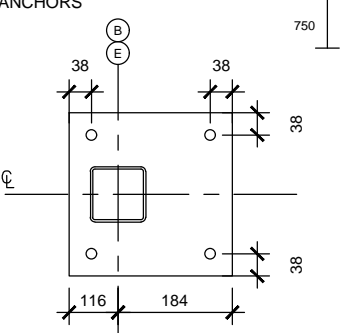
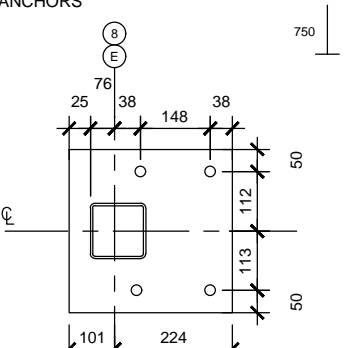
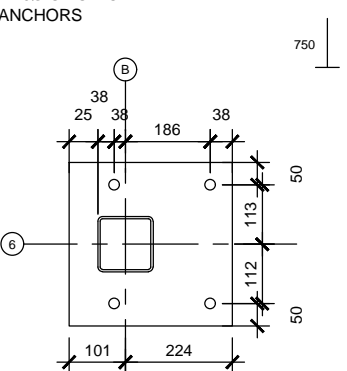
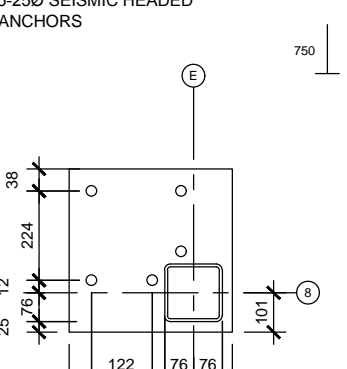
PROVIDE SLEEVES WHERE TOP BARS TERMINATE AT EDGES. PROVIDE HOOKS ON BOTTOM BARS WHERE NOTED AND AT ALL CANTILEVER ENDS.

D03-6 SLAB AND WALL OPENINGS:

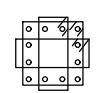
- DISPLACE BARS LATERALLY AT SLAB OPENINGS. D.O.N.O.T. C.U.T. PLACE HALF OF DISPLACED BARS EACH SIDE OF OPENING AND INFILL BETWEEN WITH BARS OF MATCHING SIZE & SPACING.
- PROVIDE 1-15M TOP AND BOTTOM MINIMUM ADDITIONAL REINFORCEMENT AROUND SLAB OPENINGS 300x300 (12"x12") OR LARGER (UNLESS NOTED). EXTEND 24 BAR DIAMETER (600mm (24" MIN.) BEYOND CORNERS.
- PROVIDE THE FOLLOWING MINIMUM ADDITIONAL REINFORCEMENT AROUND WALL OPENINGS 300x300 (12"x12

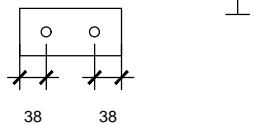
JOIST SCHEDULE	
MARK	NOTES
J1	38x235 @ 400 o.c. SPF #1/2 PRESSURE TREATED
WOOD I-JOIST NOTES: 1) WOOD I-JOIST DENOTES PRE-ENGINEERED WOOD JOIST. PRODUCTS MAY INCLUDE I-JOISTS, WOOD WEB OPEN JOISTS, WOOD CHORD/METAL WEB OPEN JOISTS. DEPTHS TO REMAIN AS NOTED. SPACING MAY BE REDUCED TO SUIT. 2) ALL MECHANICAL PENETRATIONS IN WEBS TO BE COORDINATED WITH FABRICATOR (BY CONTRACTOR) PRIOR TO SUBMISSION OF SHOP DRAWINGS. SIZES & SPACINGS OF HOLES SHOULD BE NOTED/REINFORCED AS REQUIRED. REFER TO MECH. DWGS FOR LAYOUTS. 3) JOIST SUPPLIER TO DESIGN/DETAIL/CERTIFY ALL ANCHORAGES, CLIPS, HANGERS, BRIDGING, CONNECTORS, ETC. RELATED TO THE ERECTION CAPACITY OF JOISTS. 4) PROVIDE SOLID BLOCKING AT ALL JOIST ENDS AS REQUIRED BY THE MANUFACTURER. AT SHEAR WALL LOCATIONS, TO ENSURE CONSISTENT TRANSFER OF COMPRESSIVE FORCES BETWEEN THE WALLS & FLOORS, AT ALL POST/COLUMN LOCATIONS AND CONCENTRATED LOADS, INCLUDING SIMPSON STRONG TIE HOLD DOWN ANCHORS.	

WOOD BEAM/LINTEL SCHEDULE		
MARK	SIZE	NOTES
WB1	175x418	D FIR L 24 E GLULAM TOPS OF BEAMS TO PITCH/SLOPE TO SUIT ROOF PROFILE
NOTES: 1. PROVIDE MIN. TRIPLE BUILT UP STUDS TO MATCH WALL THICKNESS EACH SIDE OF OPENINGS UN OTHERWISE. 2. FLOOR DENOTES LEVEL BELOW WHICH LINTELS OCCUR. 3. SPAN NOTED INDICATES CLEAR SIZE OF OPENING. ADD'L LENGTH TO BE PROVIDED TO ALL LINTELS TO SUIT BEARING REQUIREMENTS. 4. ALL LINTELS TO BE PROVIDED WITH MINIMUM 38mm (1 1/2") BEARING E.E. UN IN SCHEDULE. 5. SHEATHING TO BE FASTENED TO LINTELS WITH AT LEAST 2 RUNS OF FASTENERS TO THE EXTERIOR FACE OF THE LINTEL AND AT LEAST A SINGLE ROW TO THE TOP PLATES AND STUDS. FASTENERS TO CONFORM TO TABLE 9.23.3.5 OF 2012 OBC. 6. PROVIDE PLYWOOD BETWEEN PLIES OF LINTEL AS REQUIRED TO FUR LINTEL TO FULL WIDTH OF WALL STUDS. 7. LINTELS TO BE FASTENED TOGETHER AS FOLLOWS: 64mm(2 1/2")x35(1 3/8")x137(5 3/8") NAILS @ 450 (18") o.c. TOP AND BOTTOM + ADD'L PAIR @ 100mm (4") FROM ENDS. 8. THIS LINTEL SCHEDULE IS ONLY APPLICABLE TO THOSE OPENINGS THAT ALIGNED WITH OPENINGS ABOVE AND BELOW. WHERE OPENINGS DO NOT ALIGN, REFER TO PLANS FOR WOOD BEAM NOTATION. 9. ALL LINTELS TO BE TOP LOADED UN OTHERWISE.		

BASEPLATE SCHEDULE: SINGLE STOREY BUILDINGS																				
MARK	SIZE (LxWxT)	NOTES																		
BP1	300x300x25	4-250 SEISMIC HEADED ANCHORS 																		
BP2	325x325x25	4-250 SEISMIC HEADED ANCHORS 																		
BP3	325x325x25	4-250 SEISMIC HEADED ANCHORS 																		
BP4	375x375x30 (B IS SIM - MIRRORRED)	5-250 SEISMIC HEADED ANCHORS 																		
NOTES: 1. LENGTH OF ANCHORS IS EMBEDMENT LENGTH. FABRICATOR TO PROVIDE ADDITIONAL LENGTH AS REQUIRED FOR THREADS/NUTS/TOLERANCES, ETC. 2. ALL ANCHORS TO BE CAST-IN (CONCRETE) OR GROUTED INTO PLACE (MASONRY). 3. PROVIDE MIN. 25(1") THICK GROUT BED BELOW BASE PLATE UN. 4. SEISMIC HEADED ANCHORS: FOLLOWING OUTLINES MINIMUM WASHER PLATE (6W: NUT ABOVE/BELOW PLATE-TACK WELD NUT TO PLATE) SIZES FOR BOLT ENDS. <table><tr><td>16mmØ</td><td>52x52x16mm</td><td>(1/2"Ø 2"x2"x1/2")</td></tr><tr><td>19mmØ</td><td>63x63x16mm</td><td>(1/2"Ø 2 1/2"x2 1/2"x1/2")</td></tr><tr><td>25mmØ</td><td>76x76x20mm</td><td>(1"Ø 3"x2"x1/2")</td></tr><tr><td>28mmØ</td><td>82x82x20mm</td><td>(1 1/8"Ø 3 1/2"x2"x1/2")</td></tr><tr><td>32mmØ</td><td>89x89x20mm</td><td>(1 1/4"Ø 3 3/4"x2"x1/2")</td></tr><tr><td>38mmØ</td><td>102x102x20mm</td><td>(1 1/2"Ø 4"x2"x1/2")</td></tr></table> 5. ANCHOR BOLT EMBEDMENT LENGTHS HAVE BEEN DEVELOPED IN ACCORDANCE WITH CAN/CSA A23.3-04 CLAUSES 7.1.4, ANNEX D, 12.2, AND 21.2.7. DEPTHS ARE BASED ON MINIMUM EMBEDMENTS AND THE DEVELOPMENT OF THE PIER REINFORCING STEEL IN TENSION ABOVE THE CONE OF FAILURE CREATED BY THE BOLT HEAD.			16mmØ	52x52x16mm	(1/2"Ø 2"x2"x1/2")	19mmØ	63x63x16mm	(1/2"Ø 2 1/2"x2 1/2"x1/2")	25mmØ	76x76x20mm	(1"Ø 3"x2"x1/2")	28mmØ	82x82x20mm	(1 1/8"Ø 3 1/2"x2"x1/2")	32mmØ	89x89x20mm	(1 1/4"Ø 3 3/4"x2"x1/2")	38mmØ	102x102x20mm	(1 1/2"Ø 4"x2"x1/2")
16mmØ	52x52x16mm	(1/2"Ø 2"x2"x1/2")																		
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32mmØ	89x89x20mm	(1 1/4"Ø 3 3/4"x2"x1/2")																		
38mmØ	102x102x20mm	(1 1/2"Ø 4"x2"x1/2")																		

STEEL COLUMN SCHEDULE		
MARK	SIZE	NOTES
C1	HSS 152x152x13	
NOTES: 1. REFER TO PLANS FOR ELEVATIONS AND ORIENTATIONS. 2. ALL HSS COLUMNS TO BE CLASS 'C'.		

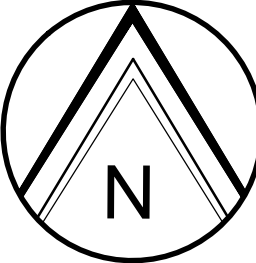
PIER SCHEDULE		
MARK	SIZE	REINFORCING
P1	600x600	12 - 20M VERT. + DOWELS 3 - 10M TIES @ 300 o.c. 
NOTES: 1. TOPS OF INTERIOR/EXTERIOR PIERS TO BE @ 300(12") BELOW THE UNDERSIDE OF THE SLAB ON GRADE. 2. PIER DIMENSIONS ARE TO BE CENTRED ON SUPPORTED COLUMNS (U.N.O.). INCREASE CONCRETE SIZE TO SUIT FOUNDATION WALL OFFSETS. 3. VERTICAL BARS IN PIERS MAY BE LAPPED WITH COMPRESSION LAPS WHERE NO BRACE/MOMENT FRAMES ARE PRESENT. WHERE BRACE/MOMENT FRAMES ARE PRESENT VERTICAL BARS ARE TO EXTEND TO FULL HEIGHT ABOVE FOOTING TO TOP OF PIERS (NO LAPS ARE PERMITTED UNLESS APPROVED BY ENGINEER IN WRITING). CLASS 'B' TENSION LAPS (MIN. 1.5 l _d) MAY BE USED WHERE HEIGHT OF PIER EXCEEDS 2400(8'-0").		

BEARING PLATE SCHEDULE (ON CONCRETE)		
MARK	SIZE	REINFORCING
BP1A	200x180x16mm	2-16mmØ HEADED ANCHORS (50x50x10 END PLATE) 
NOTES: 1. PROVIDE 25mm GROUT BED BELOW BEARING PLATES 2. ANCHORS TO BE WELDED AT UN PLATE 3. CAST PLATE/BOLTS IN CONCRETE (LEAVE 25mm FOR GROUT).		

FOOTING SCHEDULE		
MARK	SIZE (LxWxT)	NOTES
F1	1200x1200x300	15M @ 250 B.E.W. Q _{ult} ≥ 300 kPa Q _{ult} ≥ 500 kPa
F2	1000x1200x300	15M @ 250 B.E.W. Q _{ult} ≥ 300 kPa Q _{ult} ≥ 500 kPa
F3	600x300 DP	3-15M BOT. CONT. Q _{ult} ≥ 300 kPa Q _{ult} ≥ 500 kPa
F4	750x300 DP	4-15M BOT. CONT. Q _{ult} ≥ 300 kPa Q _{ult} ≥ 500 kPa
NOTES: 1. ALL FOOTINGS TO BEAR ON SUITABLE BEARING STRATA AS APPROVED BY GEOTECHNICAL ENGINEER IN ACCORDANCE WITH GEOTECHNICAL REPORT WITH MINIMUM BEARING PRESSURES NOTED IN TABLE. 2. PROVIDE MINIMUM 1500(5'-0") FROST COVER TO ALL FOOTINGS EXPOSED TO EXTERIOR CONDITIONS UN ON DRAWINGS/GEOTECHNICAL REPORT. 3. CONCRETE STRENGTH: f'c = 25 MPa(3600psi) UN. 4. TOPS OF INTERIOR FOOTINGS/WALLS/PIERS TO BE A MINIMUM OF 300(12") BELOW THE TOP OF THE SLAB ON GRADE. STEP DOWN INTERIOR FOOTINGS TO EXTERIOR FOOTINGS AS REQUIRED. FINAL BEARING ELEVATIONS TO BE BASED ON GEOTECHNICAL REPORT DATA UN. 5. FOOTING AND BOTTOM OF WALL ELEVATIONS TO BE ESTABLISHED FROM PLANS AND GEOTECHNICAL REPORT. WALLS/COLUMNS ARE TO BE TERMINATED SUCH THAT THEY ARE FOUND ON SUITABLE BEARING STRATA (WITH APPROPRIATE FROST COVER) APPROVED BY GEOTECHNICAL ENGINEER ON SITE. REFER ALSO TO GEOTECHNICAL REPORT. 6. FOOTINGS ARE TO BE LOWERED AS REQUIRED IN THE AREAS OF PITTS/SHAFTS @ FOUNDATION LEVEL SUCH THAT THE US OF THE PIT /SHAFT IS AT THE TOP OF THE FOOTING. CONTRACTOR IS RESPONSIBLE FOR COORDINATION OF ELEVATION/STEPS IN FOOTINGS. CONTRACTOR IS REQUIRED TO PREPARE FOOTING ELEVATION DRAWINGS PRIOR TO EXCAVATION.		

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(416) 211-1153

PROJECT	LINK BUILDING MILLS PHASE 3 EXPANSION
	411 COUNTRY ST., ALMONTE, ON

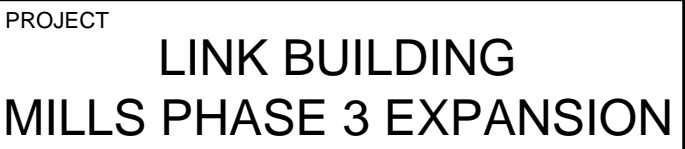
DRAWING	GENERAL NOTES AND DETAILS
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DRAWN:	A. NICHOLSON	DRAWING No.	S002
DESIGNED:	T. CAIN		
DATE:	DEC./2017		
SCALE:	N.T.S.		
PROJECT No:	17-0302		

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		STAMP		
		CLELAND JARDINE ENGINEERING LTD <small>REGISTERED COMPANY CANADA INCORPORATED (613) 591-1533</small>		
PROJECT LINK BUILDING MILLS PHASE 3 EXPANSION 411 COUNTRY ST., ALMONTE, ON				
DRAWING FOUNDATION FLOOR PLAN				
DRAWN:	A. NICHOLSON	DRAWING No.		
DESIGNED:	T. CAIN	S100		
DATE:	DEC./2017			
SCALE:	1:50			
PROJECT No:	17-0302			



STAMP



DRAWING

FOUNDATION FLOOR
PLAN

S100

PLOTTED ON: 21-Feb-2018

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ROOF NOTES:

SEE ELEVATIONS ON S300 FOR CONFIGURATION/DESIGN LOADS IN MOMENT FRAMES.

100 DENOTES SEISMIC LOADS IN DIAPHRAGM CHORDS (FOR CONNECTION DESIGN). FORCES GIVEN HAVE NOT BEEN AMPLIFIED BY RD. LOAD ARE IN kN. (LOADS CAN BE POSITIVE OR NEGATIVE)

► DENOTES MOMENT/SHEAR CONNECTION REFER TO PLANS FOR LOCATIONS AND DESIGN LOAD.

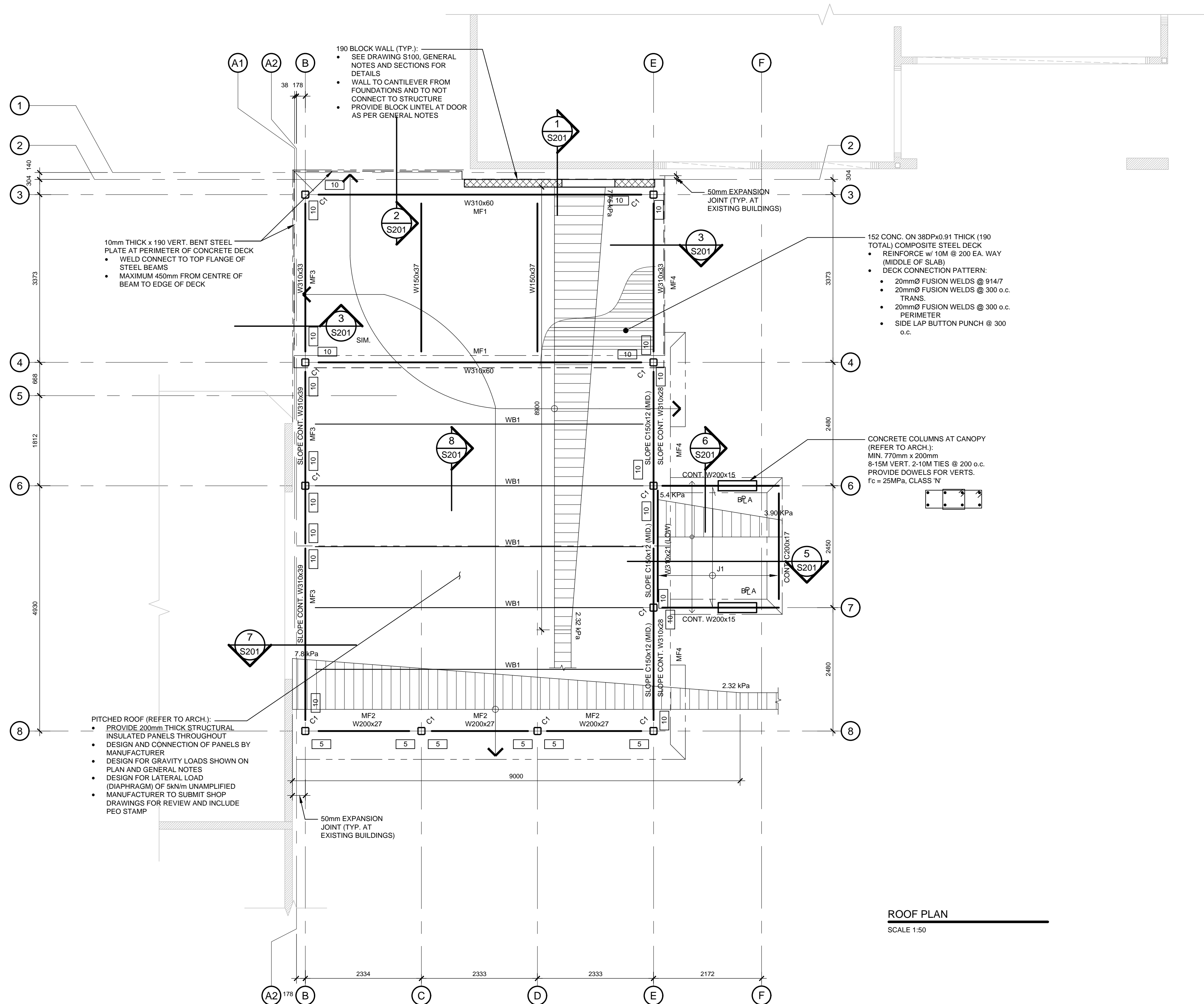
DECK CONNECTION PATTERN ROOF:

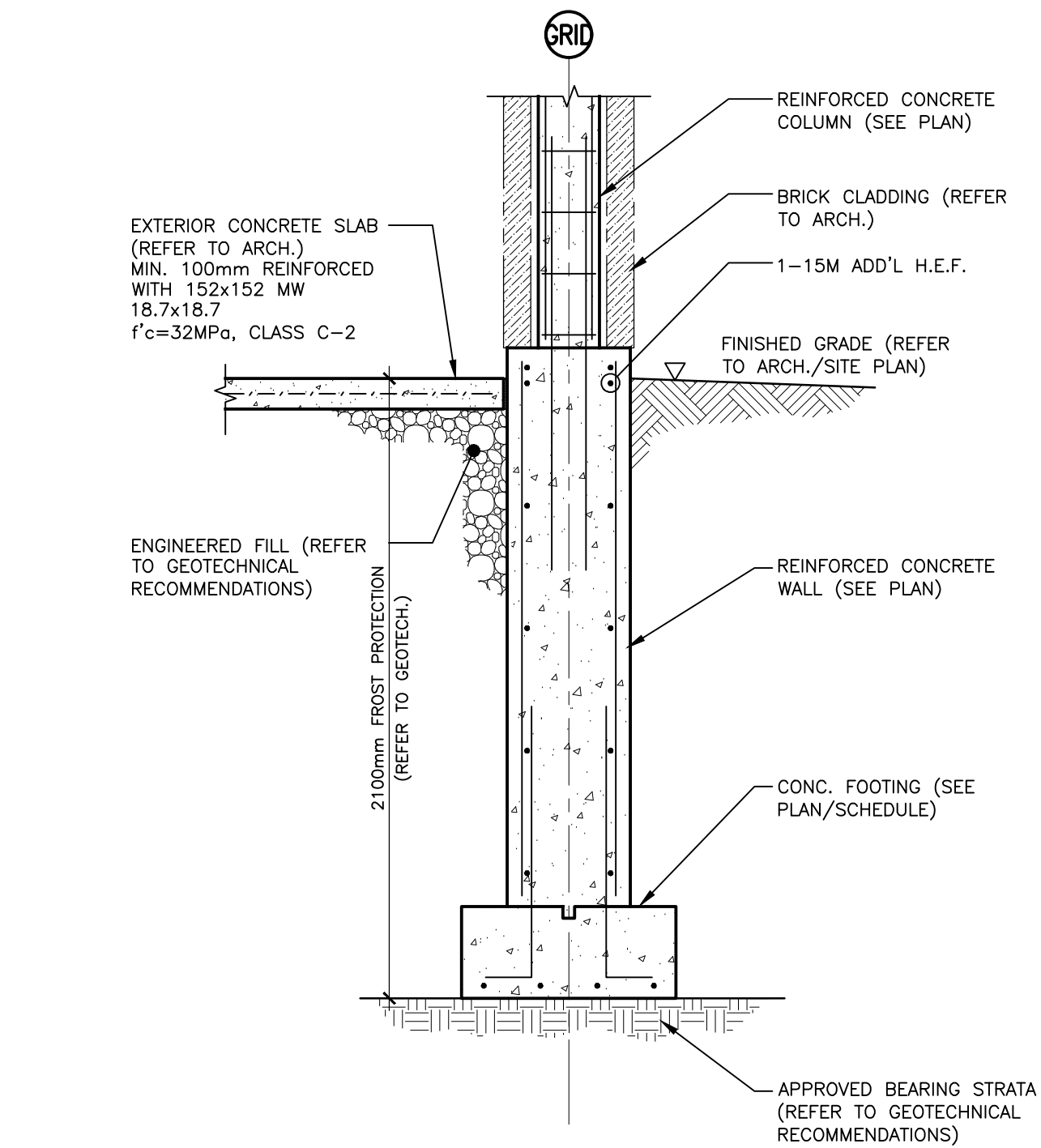
- 20mmØ FUSION WELDS @ 914/ 7
- 20mmØ FUSION WELDS @ 300mm o.c. TRANS
- 20mmØ FUSION WELDS @ 300mm o.c. PERIMETER
- SIDE LAP BUTTON PUNCH @ 300mm o.c.

ALL COMPOSITE STEEL DECK TO BE REINFORCED w/ 10M @ 200 EACH WAY (152mm CONC. ON 38mm DP STEEL DECK (150mm TOTAL))

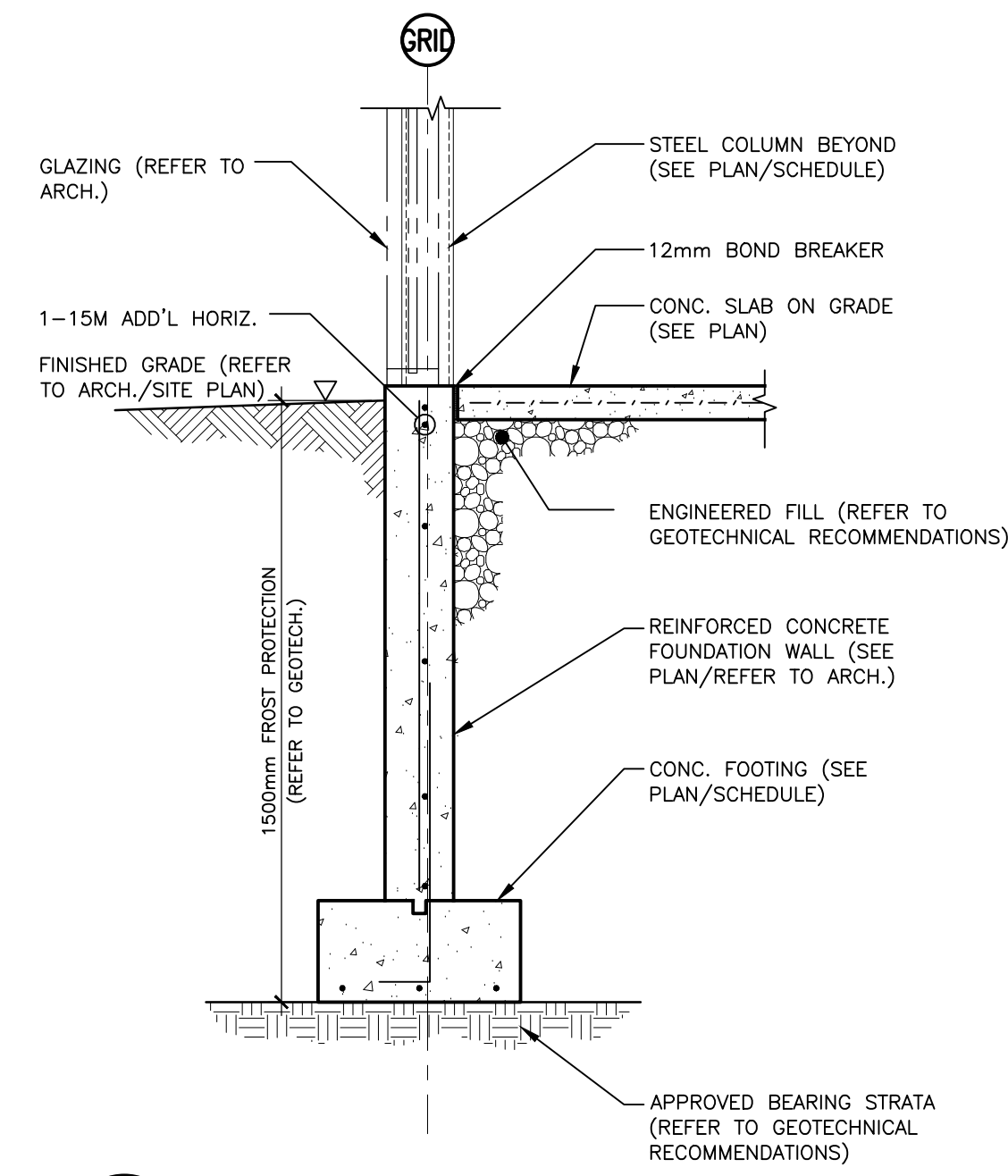
ROOF T.&G. DIAPHRAGM SHEATHING (BLOCKED DIAPHRAGM):

16mm T.&G. PLYWOOD (EXTERIOR GRADE FOR ROOF) NAILED WITH 64mm x 3.25mmØ COMMON WIRE NAILS @ 150mm o.c. @ PANEL EDGE @ 300mm o.c. AT PANEL POINTS 64mmx3.25mmØ COMMON WIRE NAILS @ 150mm o.c. AT DIAPHRAGM BOUNDARIES

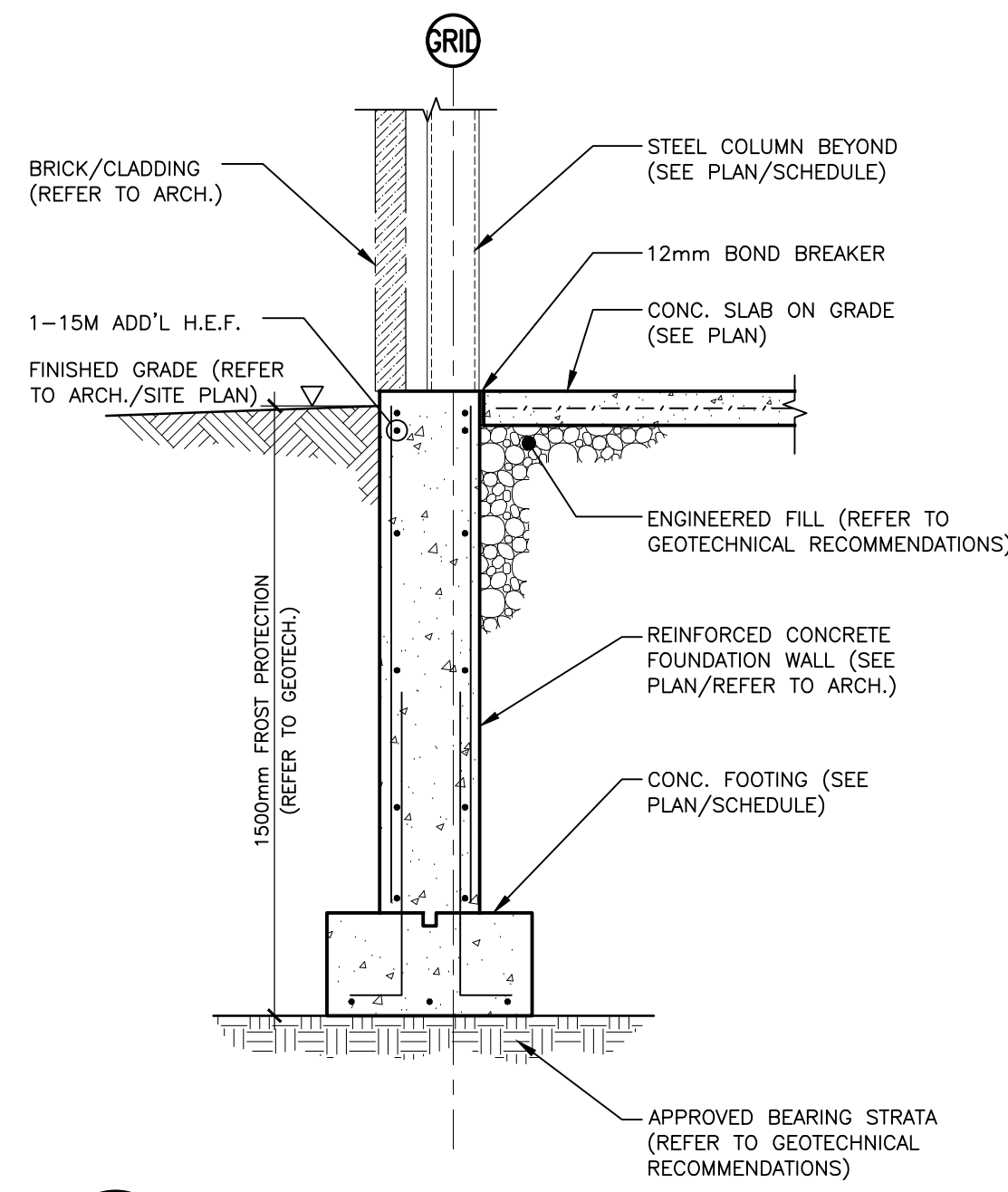




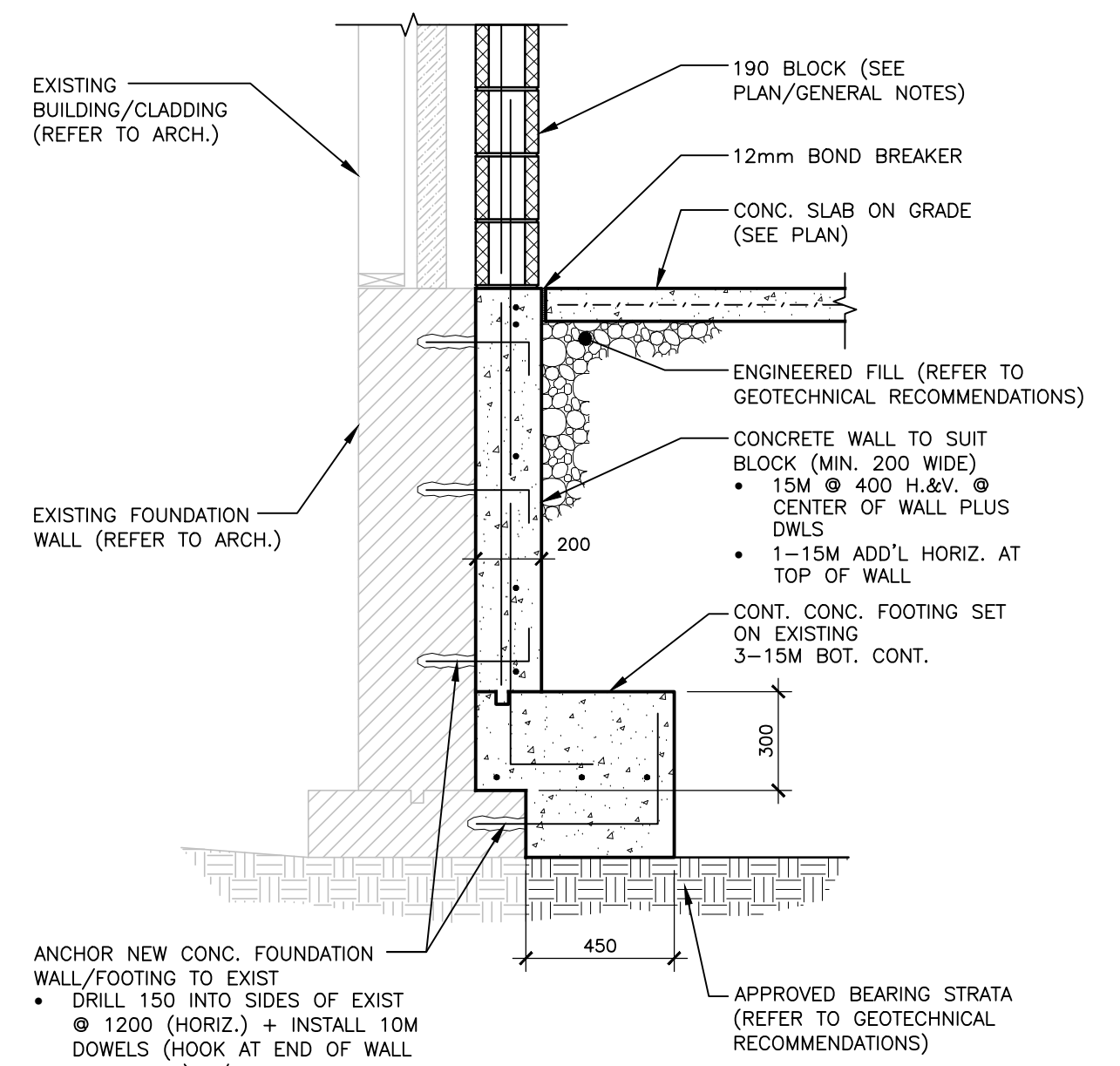
4 SECTION: CANOPY FOUNDATIONS
S200 SCALE 1:20



3 SECTION: TYP. FOUNDATION @ GLAZING
S200 SCALE 1:20

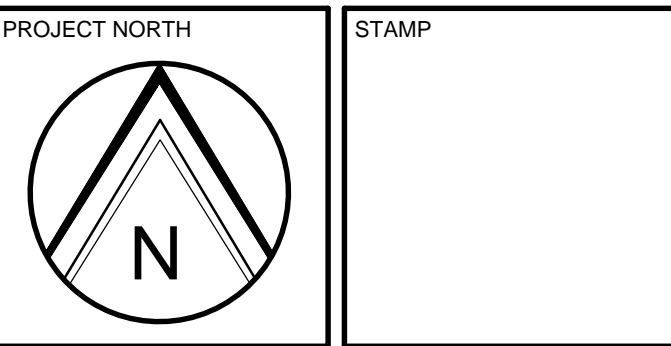


2 SECTION: TYP. FOUNDATION @ BRICK
S200 SCALE 1:20



1 SECTION: BLOCK @ EXISTING INTERFACE
S200 SCALE 1:20

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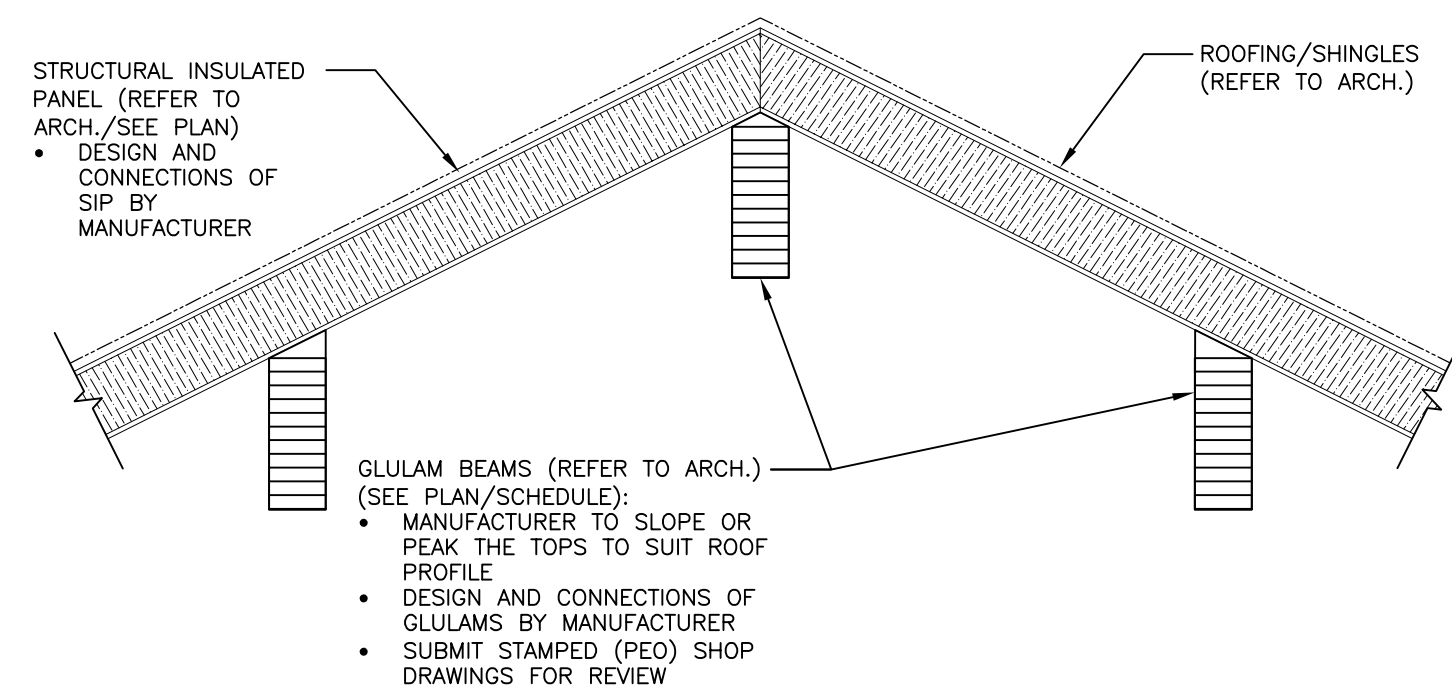
PROJECT
**LINK BUILDING
MILLS PHASE 3 EXPANSION**

411 COUNTRY ST., ALMONTE, ON

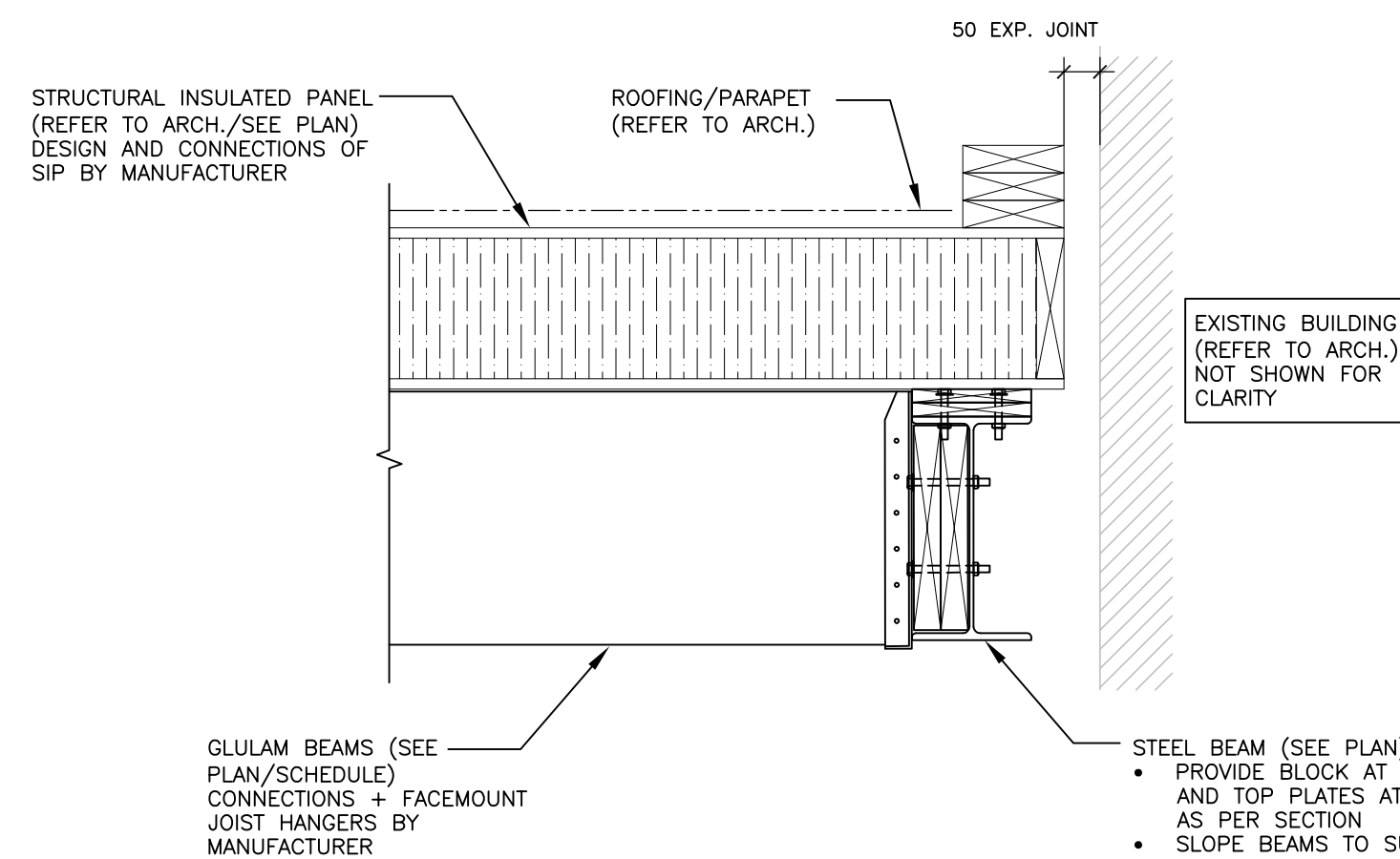
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SECTIONS

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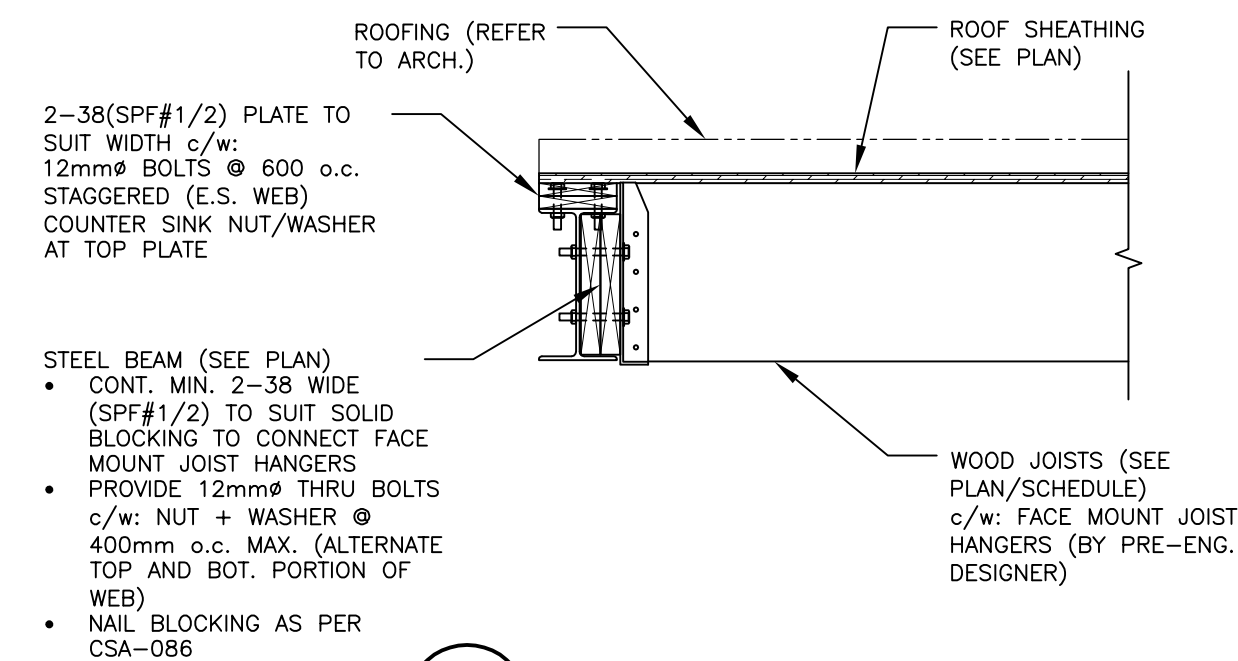
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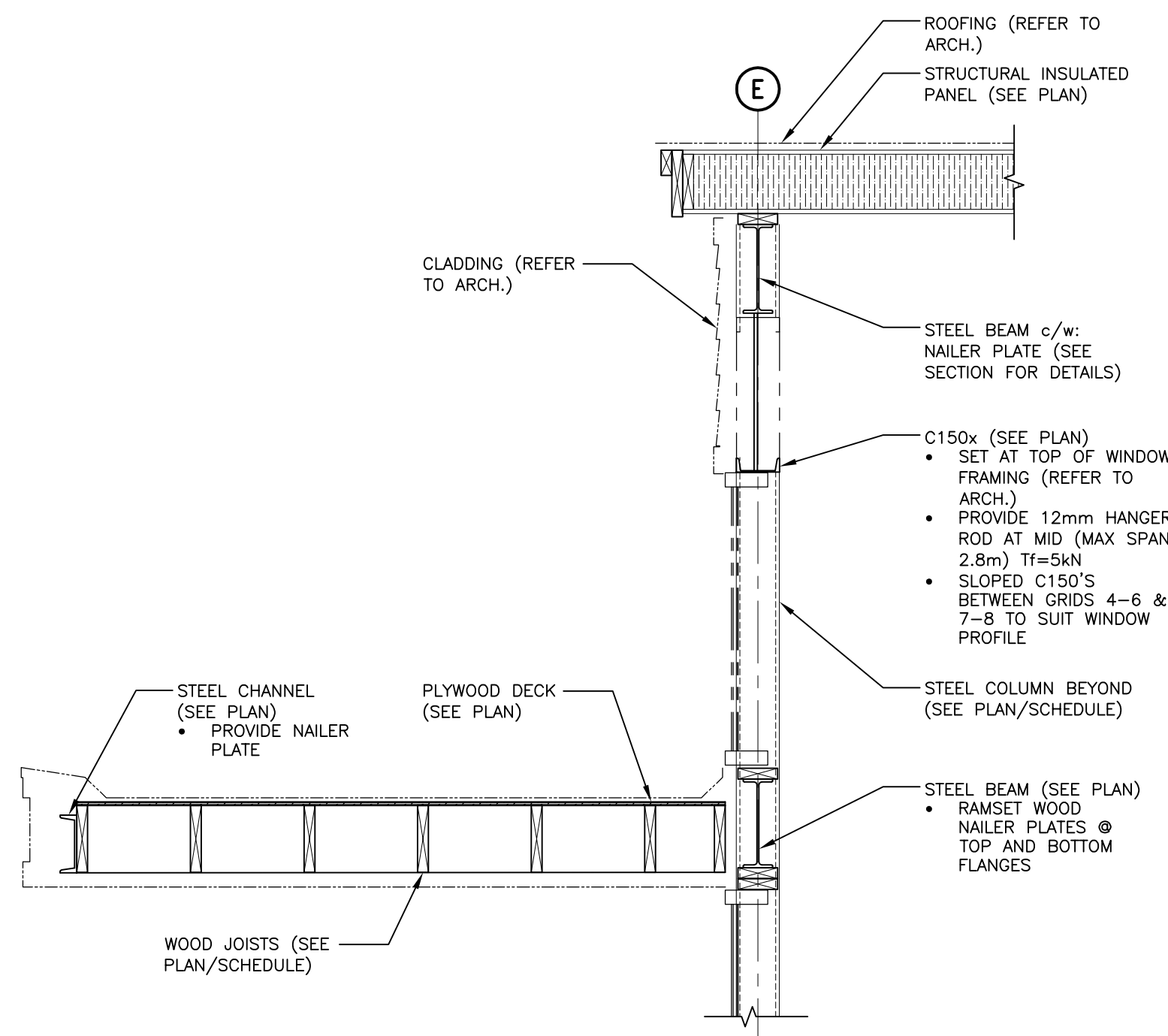
8 SECTION: PITCHED ROOF AT PEAK
S201 SCALE 1:20



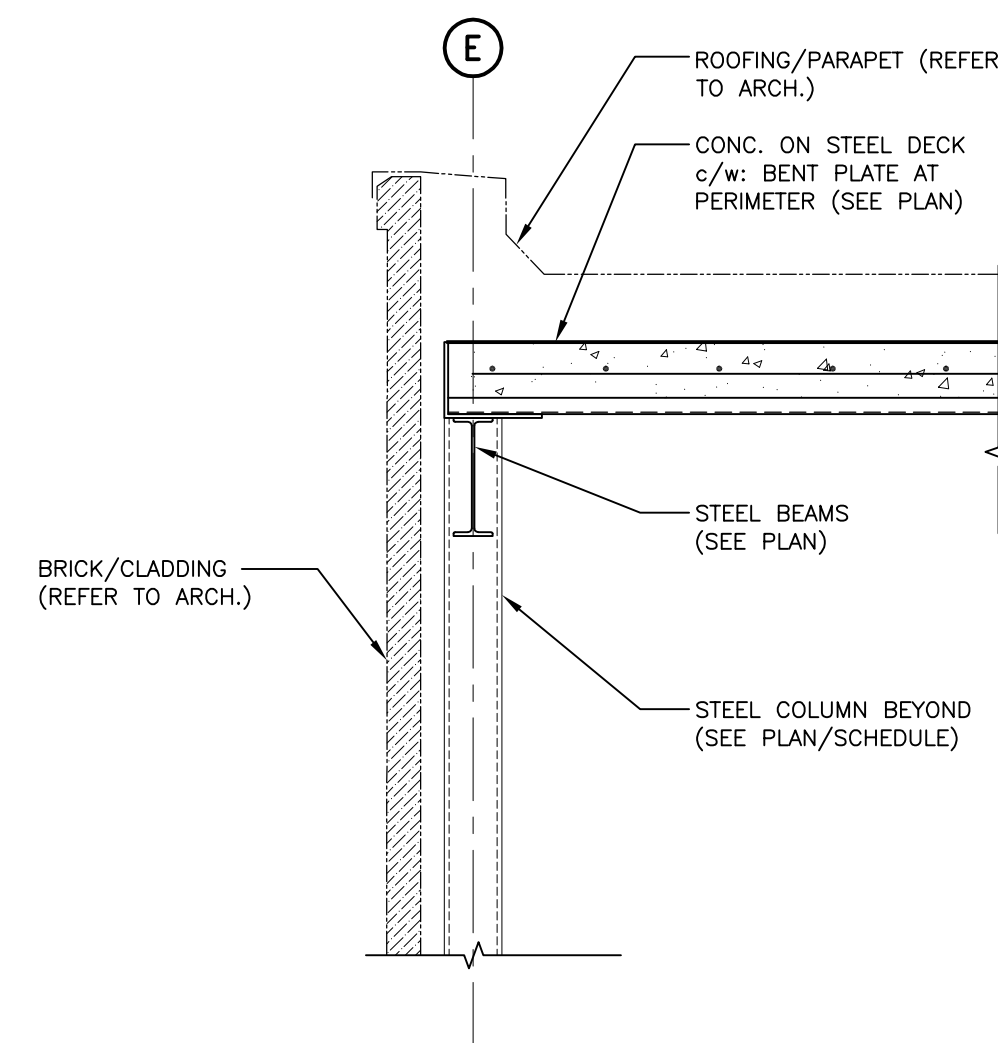
7 SECTION: PITCHED ROOF AT EXISTING
S201 SCALE 1:10



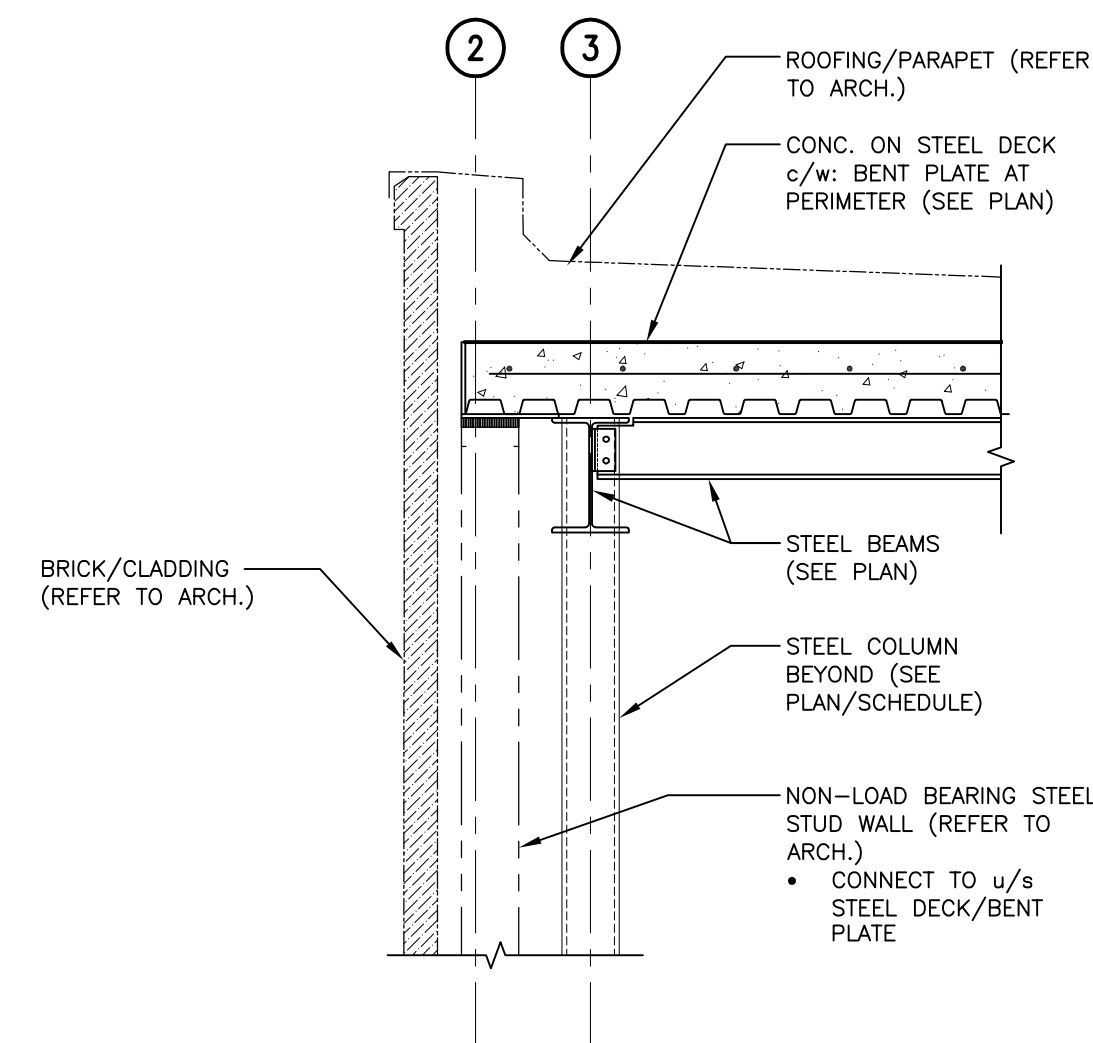
6 SECTION: CANOPY AT BEAM
S201 SCALE 1:10



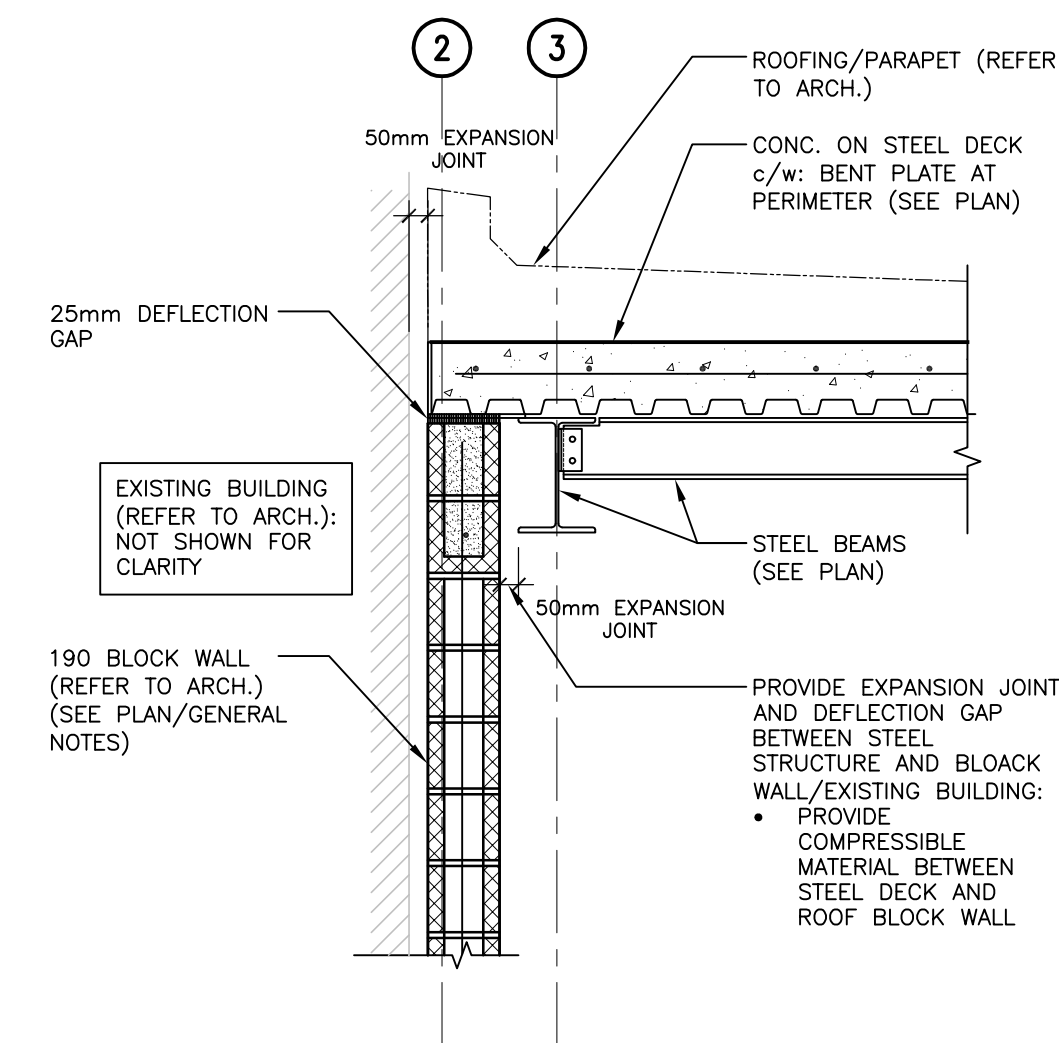
5 SECTION: CANOPY FRAMING
S201 SCALE 1:20



3 SECTION: CONCRETE ON DECK (GRID E)
S201 SCALE 1:20

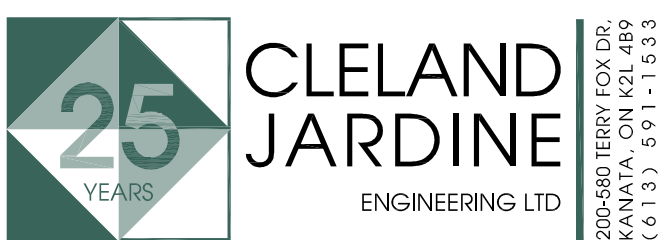
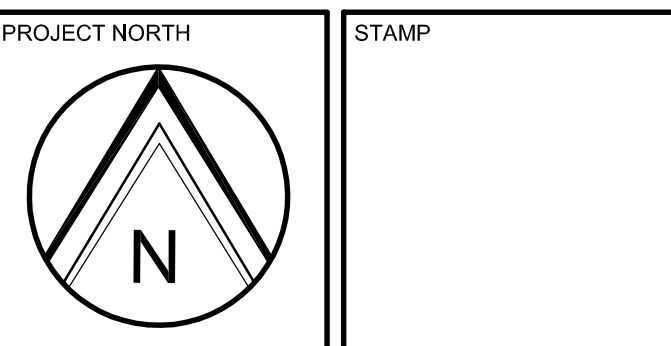


2 SECTION: CONCRETE ON DECK (GRID 3)
S201 SCALE 1:20



1 SECTION: PHASE 2 INTERFACE (BLOCK AND STRUCTURE)
S201 SCALE 1:20

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PROJECT
**LINK BUILDING
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411 COUNTRY ST., ALMONTE, ON

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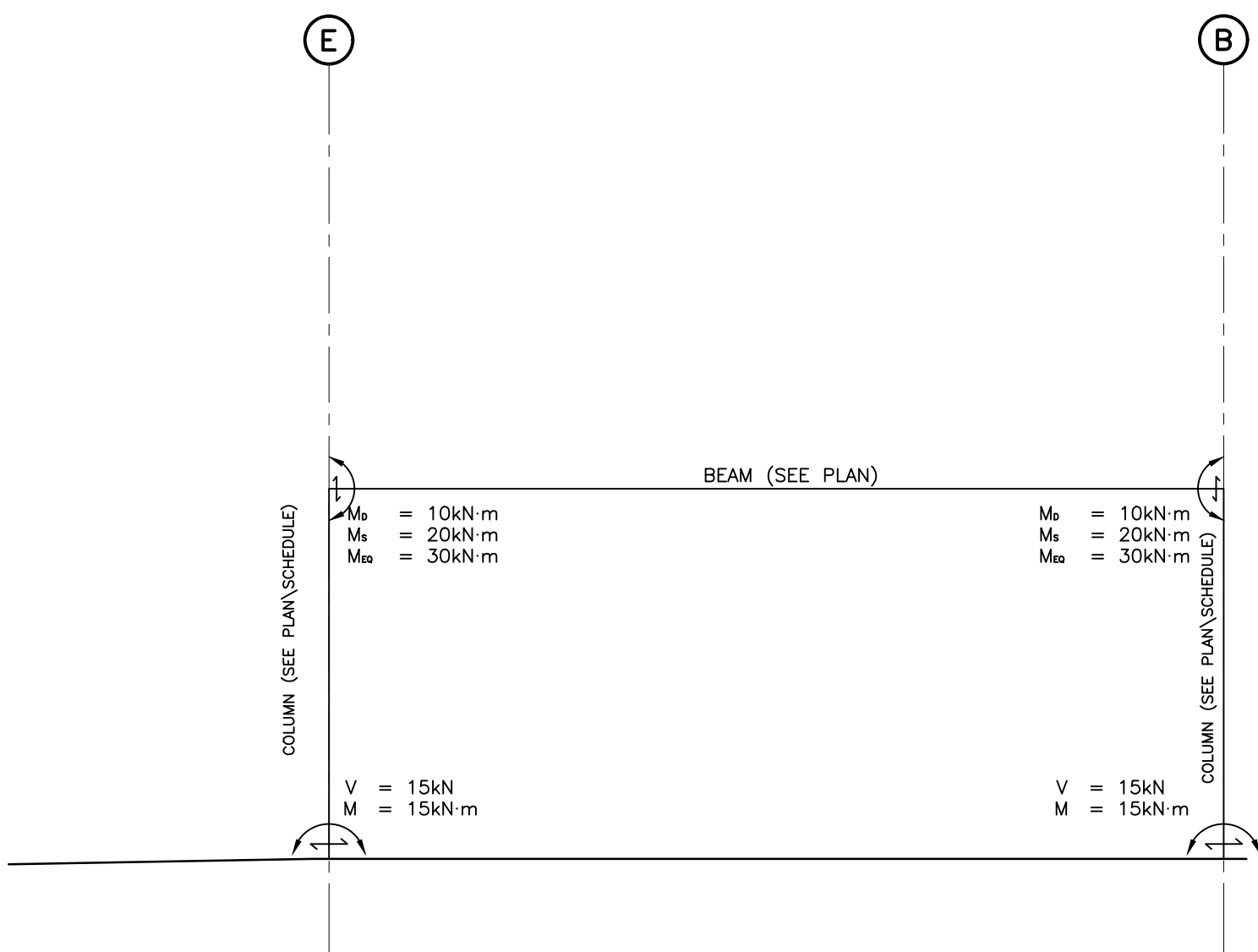
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S201

MOMENT FRAME NOTES:

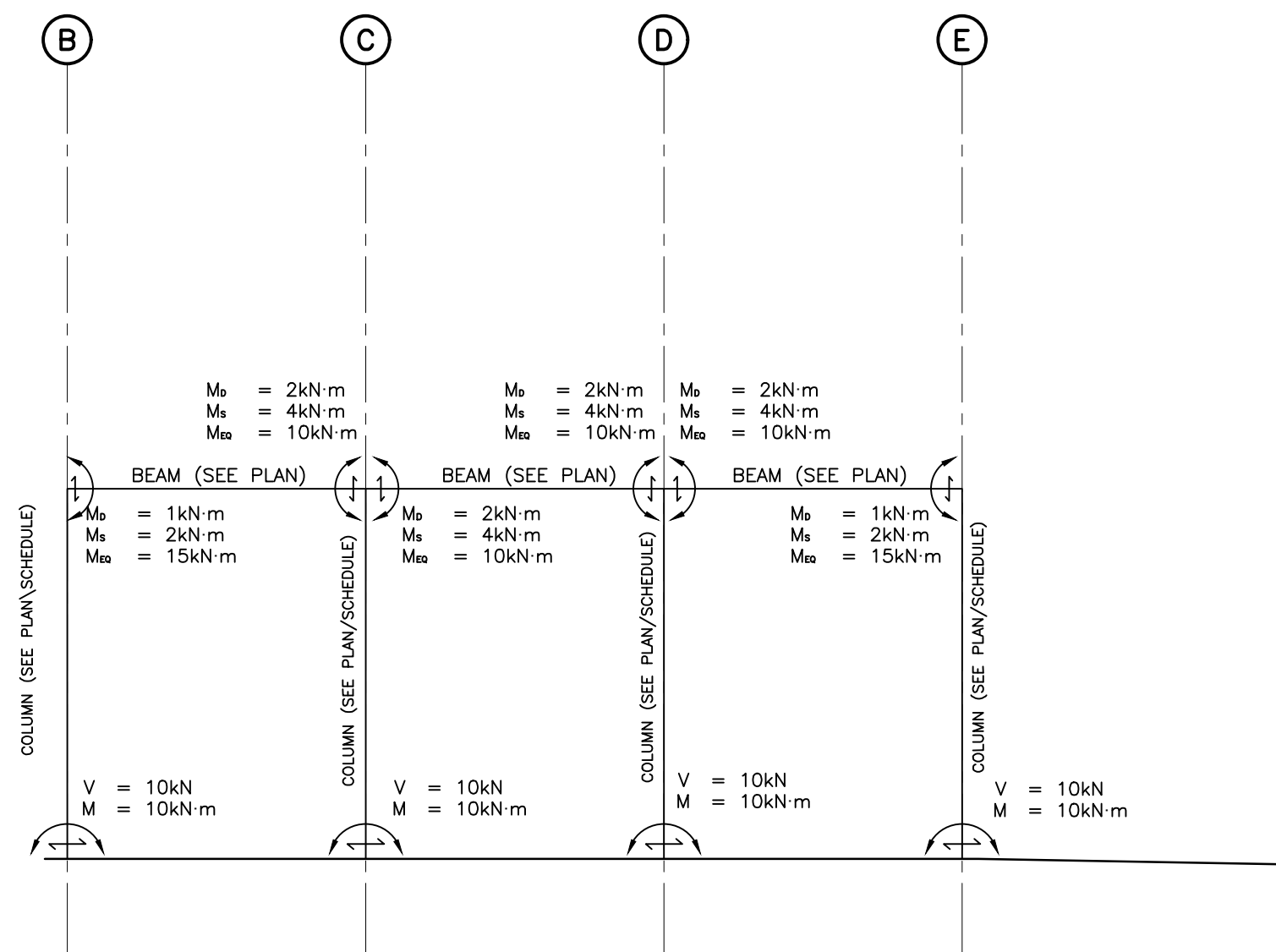
- TYPE: CONVENTIONAL STEEL MOMENT FRAME
- BEAM/COLUMN CONNECTIONS TO BE DESIGNED IN ACCORDANCE WITH CLAUSE 27.11 OF S16-09
- LOADING TO BE COMBINED AS PER 2012 OBC/S16-09 AS REQUIRED
- ALL FORCES AND MOMENTS TO BE MULTIPLIED BY $R_d=1.5$
- DESIGN SHEAR CONNECTION FROM RESULTING UNIFORMLY DISTRIBUTED LOADING GIVING FLEXURAL CAPACITY IF SIMPLY SUPPORTED

REFER TO PLANS AND SCHEDULES FOR BEAM, COLUMN AND BASE PLATE SIZES



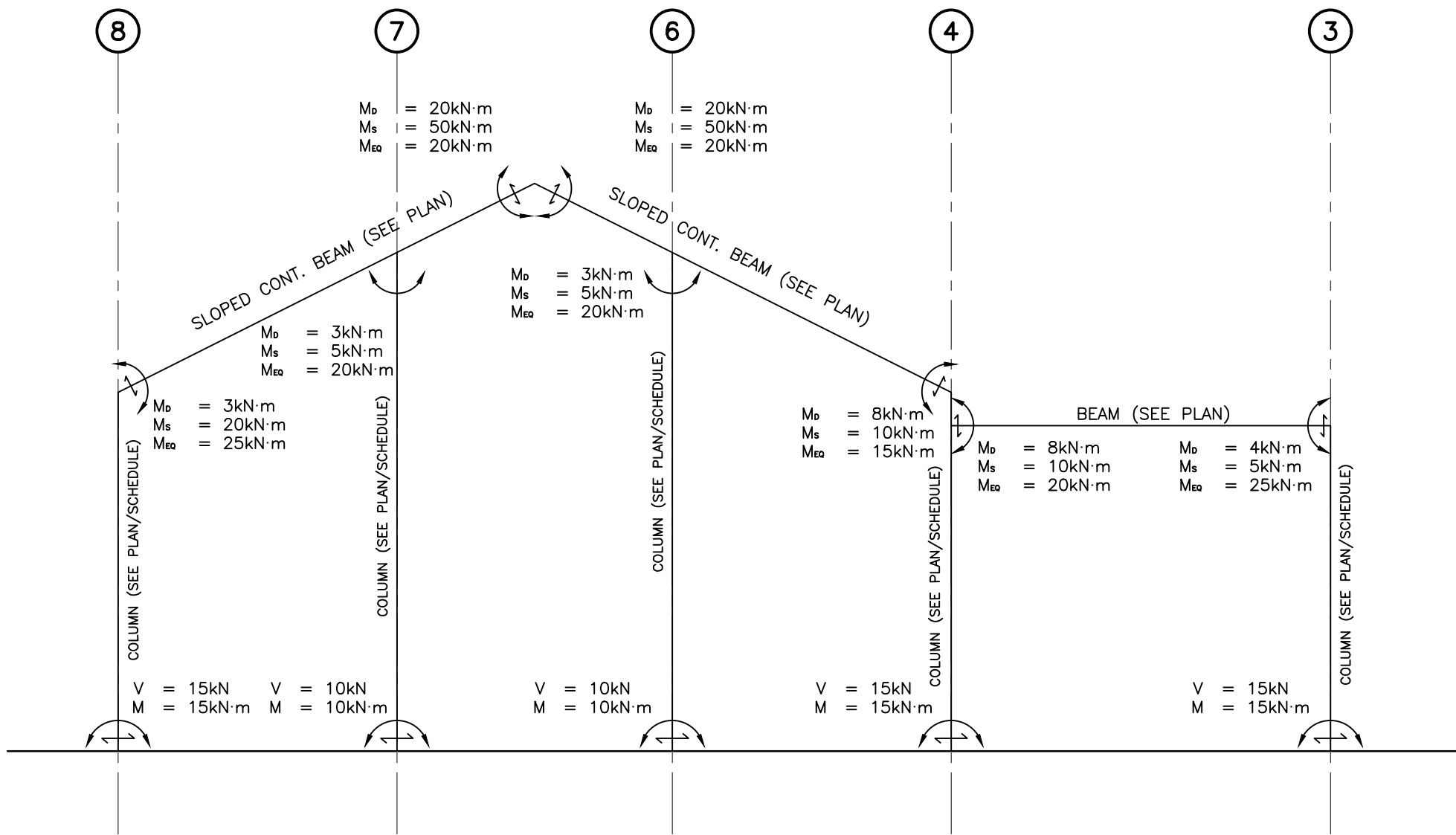
ELEVATION: MOMENT FRAME 1 (2 TOTAL - GRIDLINE 3&4)

SCALE 1:50



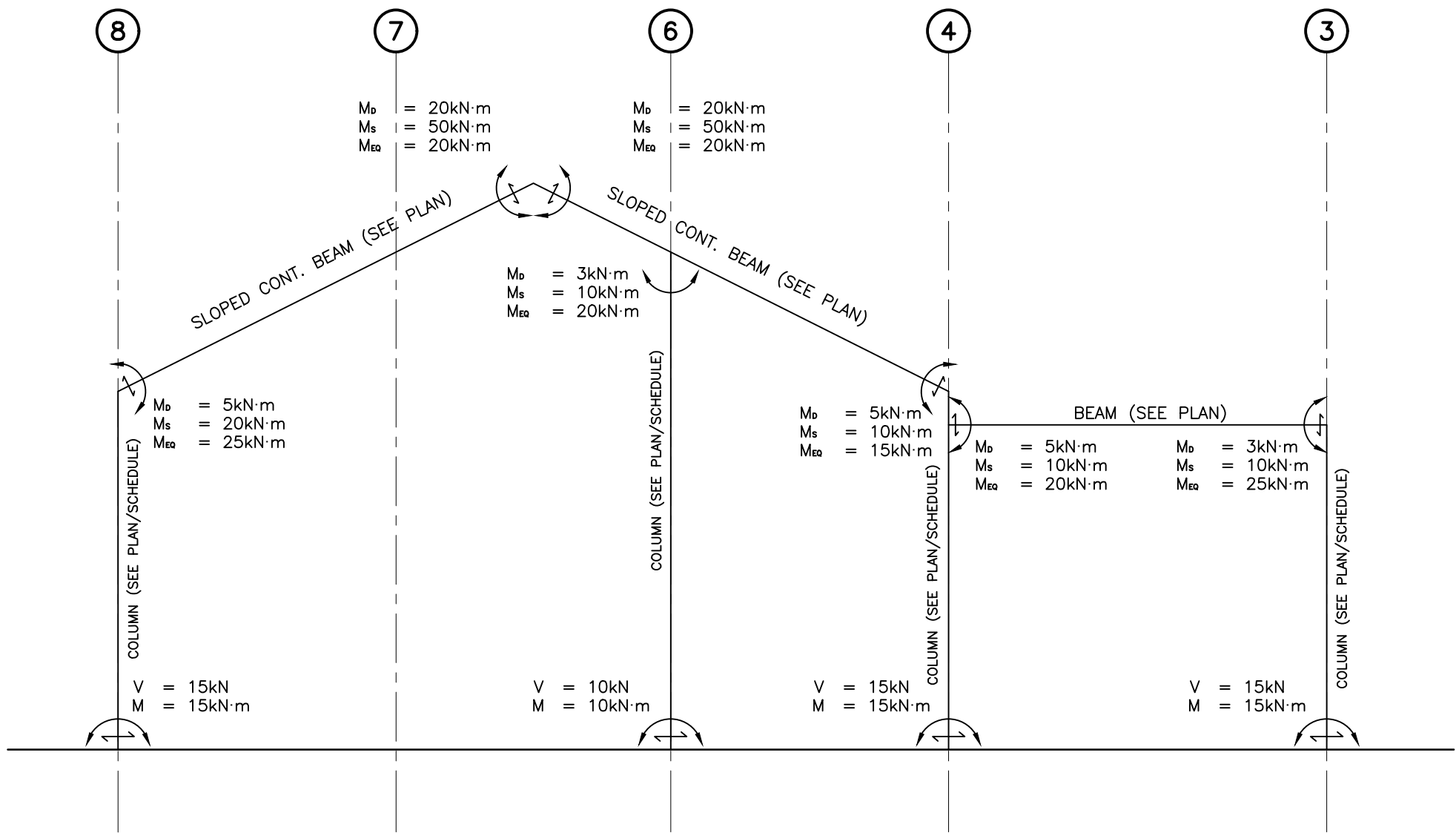
ELEVATION: MOMENT FRAME 2 (GRIDLINE 8)

SCALE 1:50



ELEVATION: MOMENT FRAME 4 (GRIDLINE E)

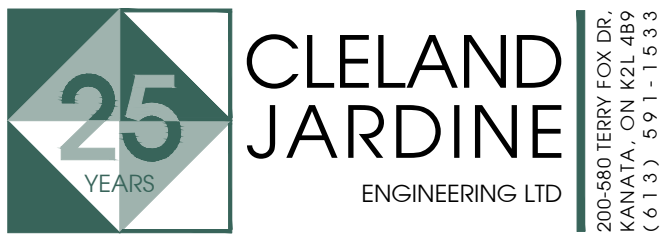
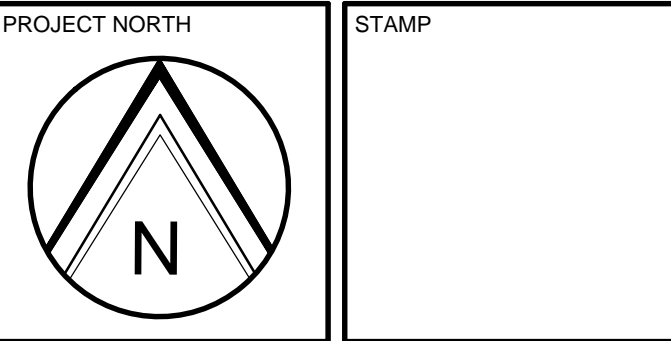
SCALE 1:50



ELEVATION: MOMENT FRAME 3 (GRIDLINE B)

SCALE 1:50

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LINK BUILDING
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S300